Design of Nuclear Safety-Related Structures for Impactive and Impulsive Loads using ACI CODE-349 and ASME Section III, Division 2 Provisions— Guide

Reported by ACI Committees 349 and 359



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PREFACE

This guide is the work of the ACI 349/359 Task Group on Impulsive and Impactive Loading that established a proposal of updated design provisions for both ACI CODE-349 and ASME BPVC Section III, Division 2. The provisions of this guide are nonbinding on ACI CODE-349-23 Appendix F or ASME BPVC Section III, Division 2:2011 and may change if implemented.

This guide gives the genesis of the provisions that are proposed to be incorporated into these code documents and further explains why these provisions were adopted. Furthermore, the primary mandate of this guide was to align both aforementioned codes and to bring these codes up to date with current industry practices, using current references for the provisions of both code documents. This guide serves as an independent report of the task group's work to the code committees. This guide is issued as a detailed commentary to the provisions of ACI CODE-349-23 Appendix F or ASME BPVC Section III, Division 2. The language of ACI CODE-349-13 and ASME BPVC Section III, Division 2:2011 form the starting point for the proposed nonbinding revisions already incorporated in the first two columns of this guide. The last column does not provide proposed changes but provides explanations of why such changes are proposed.

Keywords: blast; hard impact; impact; impulse; nuclear structure; perforation; scabbing; soft impact; structural analysis; structural design.



	NOTATION AND DEFINITIONS	<i>p</i> =	pressure (Section RF.6.1); perimeter of missile
Netetien		р _	cross section (Section RF./.2.1)
NOLA		$K_{avail} =$	available resistance
A_d	= area of diagonal reinforcement	$K_{DL} =$	dead load resistance
A_g	= gross area of concrete section	$R_{m1} = D$	resistance required at F_1
A_s	= area of steel reinforcement	$R_{m2} = D$	resistance required at F_2
A_{sh}	= area of noop reinforcement	$R_u =$	ultimate resistance
a_g	= aggregate size of concrete	r =	percentage of reinforcement described by the
b	= width of compression face of member		percentage each way in each face
С	axis	r_{θ} =	Fig. RF.3.3)
C_r	= reinforcing bar spacing	$s_h =$	spacing of hoops
D	= effective missile diameter	t =	total section thickness
d	= distance from extreme compression fiber to	$t_d =$	impact duration
	centroid of longitudinal tension reinforcement	$t_p =$	perforation thickness
	(Section F.2.3); effective diameter or the diameter	$t_s =$	scabbing thickness
	of the hard/rigid part if the missile contains flexible	$\dot{u} =$	reference velocity
	part (Section RF.7.2.1)	$\dot{u}_{max} =$	impact velocity
d_e	= distance between the front face and reinforcement	$V_d =$	direct shear capacity
d_{load}	= diameter of loaded area	$V_u =$	factored shear force at section
e_b	= eccentricity corresponding to balanced strain	v =	missile impact velocity
_	conditions	W =	weight of missile
F	= impact force demand for high-mass, low-velocity	$X_m =$	maximum acceptable displacement
	impact (Section F.6.3); average value of the time-	$X_y =$	effective yield point
	dependent force resultant of the missile (Section	<i>x</i> =	penetration depth of missile
	RF.6.4)	$Y_j =$	jet impingement load, or related internal moments
F_1	= maximum impulsive loading		and forces, on the structure generated by a postu-
F_2	= approximately constant impulse loading during Δt		lated pipe break
F_e	= effective load for a single degree of freedom	$Y_m =$	missile impact load, or related internal moments
	(SDOF) model (Section RF.6.1)		and forces, on the structure generated by a postu-
F_p	= static punching capacity of a slab		lated pipe break, such as pipe whip
F_t	= resultant force	$Y_r =$	loads, or related internal moments and forces, on
f_c'	= concrete compressive strength		the structure generated by the reaction of the broken
f_{dc}	= dynamic compressive strength		pipe during a postulated break
f_{ds}	= dynamic stress capacity of steel reinforcement	$\alpha =$	angle defining orientation of reinforcement
f_u	= ultimate strength	$\Delta t =$	time interval
f_y	= specified yield strength of reinforcement	$\varepsilon_{cu} =$	maximum concrete compression strain
H_0	= target thickness	ϕ =	strength reduction factor
h	= overall thickness or height of member	$\phi(x,y)=$	deformed shape function
Ι	= total impulse	μ =	ductility ratio
Icr	= moment of inertia of cracked section transformed	μ_d =	permissible ductility ratio
	to concrete	ho =	ratio of A_s to bd (Section F.4.2); concrete density
I_g	= moment of inertia of gross concrete section about		(Section RF.7.2.1)
	centroidal axis, neglecting reinforcement	$ ho_p$ =	average percentage of reinforcement on the
Κ	= value of F_t to cause unit deflection at point of appli-		tensioned face
	cation of resultant force (Section RF.6.1); concrete	σ =	principal stress
	penetrability factor (Section RF.7.2.1)	σ_e =	effective stress
K_e	= elastic stiffness	ψ =	cross-section curvature (Attachment B)
k	= stiffness of impactor (Section F.6.3)		
L_d	= development length	Definit	tions
ℓ_h	= unsupported length	abno	rmal, extreme environmental, and abnormal
М	= mass of missile	and ex	treme environmental load categories—structural
M_e	= equivalent mass	membe	rs designed to resist impactive and impulsive loads
m	= mass per unit area (Section RF.6.1); mass of the	and dynamic effects are structures in the abnormal, extreme	
	impactor (Section F.6.3)	environmental, and abnormal and extreme environme	

N= missile nose shape factor

(aci)

- = nominal axial strength at balanced strain conditions P_b $P_{n(max)}$ = design axial strength
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categories, which can have permanent, plastic deformations.

F.3.3 and F.3.4 as invoked either by the ACI CODE-349 or

component damage levels-acceptable damage levels in