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## Monitoring for Cold Weather Concreting

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WEB SESSIONS



**James Baty** is the Managing Director for the Concrete Foundations Association and Technical Director for the Tilt-Up Concrete Association. He has a career-long emphasis on thermal dynamics and behavior of concrete as well as concrete construction and design education. As a director for the CFA, he overseen the research efforts of multiple projects related to concrete performance.



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WEB SESSIONS

## Cold Weather Affects on Residential Concrete Foundation Walls

Presented by:  
James R. Baty II, Managing Director



## Research Results

Phase I, II and III conducted  
November, 2002 to July 2003

- Testing Lab Cylinders
- Simulated Field Walls
- Petrographic Investigation



## The Research Team

- ◆ Research Leader
  - ◆ Brad Barnes P.E., North Central Engineering, Ltd., Canton, OH
- ◆ Suppliers
  - ◆ Master Builders, Inc., Cleveland, OH
  - ◆ Osborne-Medina Concrete, Medina, OH
  - ◆ Con-Cure Corporation, St. Louis, MO
  - ◆ Cemstone Concrete, Minneapolis, MN
- ◆ Contractors
  - ◆ Lavy Concrete Construction, Piqua, OH
  - ◆ Modern Poured Walls, LaGrange, OH
  - ◆ Tri-County Excavating Inc., Richfield, OH
  - ◆ Dependent Foundations, Brighton, MI
  - ◆ Van Wyks, Inc., Waldo, WI
  - ◆ JC Concrete Inc., Berrien Springs, MI
  - ◆ Martinson Construction, Waterloo, IA

## A Response to the Market...

2001 – Ohio  
Over a dozen projects shut down by inspectors due to "cold weather".

2002 – North Carolina  
Contractors are not permitted to place concrete below 50° F.

2001 – Michigan  
Contractors routinely and successfully place at sub-freezing temperatures without restrictions.

Policy #099 – Anchorage, Alaska  
Concrete strength to reach 1,500psi at 24 hours...submit documentation verifying results  
Tenting must be provided until concrete reaches 1,500psi when temps fall below 35° F.

## Precipice

- ◆ Need to understand how “cold weather” impacts the residential concrete foundation wall industry.
- ◆ Existing codes – Protective measures must be taken; Empirical evidence – they may not be necessary or even helpful.
- ◆ Variations in “local” mixture performance mandates a need for method of validation of in place strengths.
- ◆ What constitutes cold weather is even debatable. In other words...

## The Research Program

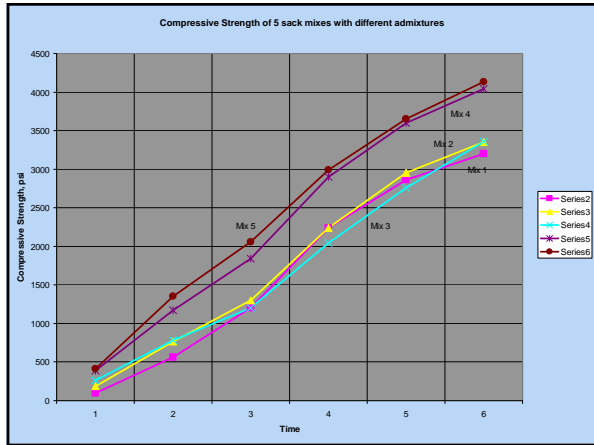
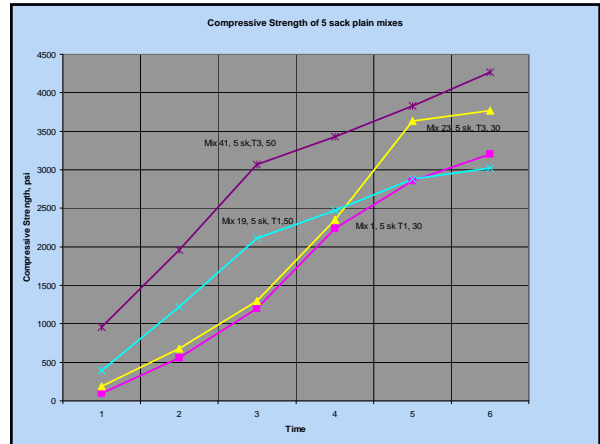
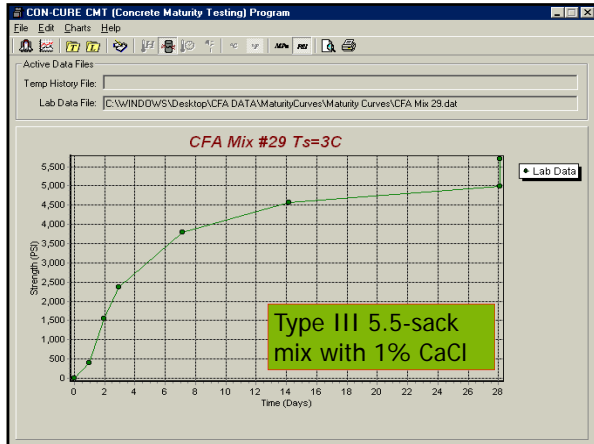
- ◆ Phase I: Laboratory Chiller Cylinders
  - ◆ 36 mix designs
  - ◆ 44 maturity curves
  - ◆ over 650 cylinders cast and tested
  - ◆ Wide range of mixes from very “lean” to very “rich”
  - ◆ Two different temperatures, 30°F and 50°F
- ◆ Phase II: Field Mock-Up Walls
  - ◆ 6 selected mix designs
  - ◆ 24-hour blanketed vs. unblanketed
- ◆ Phase III: Laboratory Petrographic Studies

## Mix Design Variation

Qty of Cement (Sacks per CY)	Cement Type	Admixtures	Curing Temperature
5	I	None	30°F
5.5		1% Calcium	
6	III	2% Calcium	50°F
6.5		1% Calcium with MRWR	
		Non-Chloride Accelerator (NCA)	

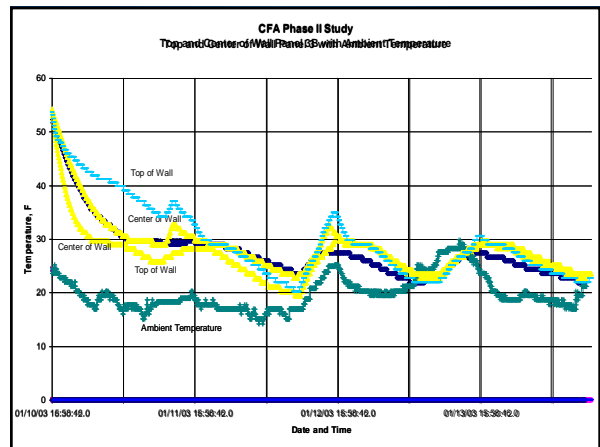






Mix Desc.	Data Set	1 Day (~30 hrs.)	2 Day (~48 hrs.)	3 Day (~73 hrs.)	7 Day (~170 hrs.)	28 Day (~702 hrs.)	180 Day
5-sack Type I 2% CaCl	Uncov.	330	560	1040	1740	3410	5530
	Cov. §	340	600	1060	1790*	3290*	5765
	Matur.	230	500	810	1450	-	-
5.5-sack Type I 2% CaCl	Uncov.	410	600	1020	1650	3460	5695
	Cov. §	400	590	1000	1815*	3520*	6250
	Matur.	280	580	940	1690	-	-
6-sack Type I 2% CaCl	Uncov.	510	700	1160	1840	3800	6955
	Cov. §	520	740	1280	2005*	3605*	-
	Matur.	560	950	1350	2100	-	-
5-sack Type III 1% CaCl	Uncov.	500	840	1350	1750	3150	5750
	Cov. §	510	870	1470	2035*	3270*	5450
	Matur.	450	1050	1560	2070	-	-
5.5-sack Type III 1% CaCl	Uncov.	990	1320	1840	2220	4140	5550
	Cov. §	990	1370	2110	2710*	4180*	5750
	Matur.	550	1530	2280	2990	-	-
6-sack Type III 1% CaCl	Uncov.	1400	1870	2500	3030	5250	6500
	Cov. §	1550	1970	2620	3360*	4965*	6850
	Matur.	1180	2060	2760	3820	-	-

\* Average strength of 2 cores taken from the top one foot and center of the same wall panel.  
§ Covered with a curing blanket for first 18 hours after it was placed.



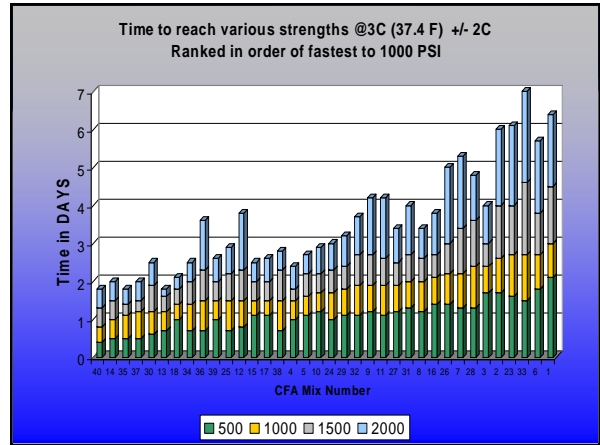
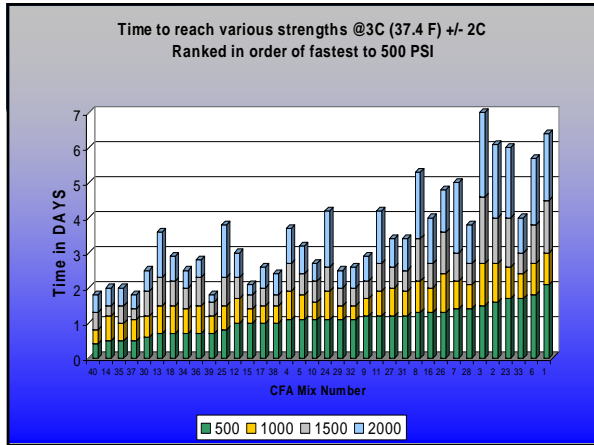
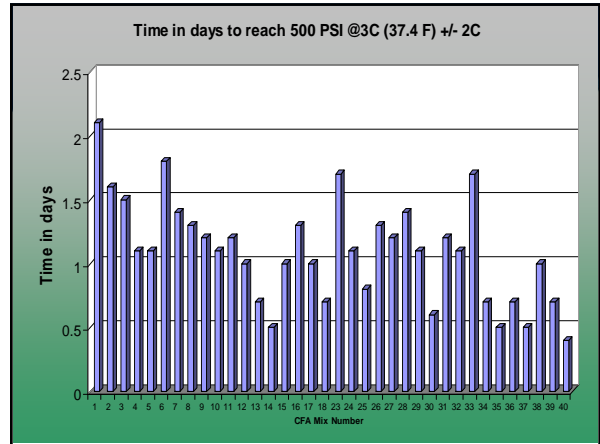
**CFA Cold Weather Testing  
Phase II Study**

**Petrographic Examination Summary**

Panel/Mix ID	Microfracturing <sup>1,2</sup>			Effect on Cement Paste <sup>3,4</sup>		
	Severe	Moderate	Mild	Severe	Moderate	Mild
3	3/8"	1-1/2"	Entire Thickness	1/16"	1-1/4"	N/A
3B	N/A	N/A	N/A	N/A	N/A	1/16"
8	3/8"	1-1/2"	2-1/2"	1/32"	1-1/4"	N/A
8B	N/A	N/A	N/A	N/A	N/A	Superficial
13	3/8"	1-1/8"	1-3/4"	1/32"	3/8"	N/A
24	3/8"	1-3/4"	2-3/4"	1/32"	3/8"	N/A
24B	N/A	N/A	N/A	N/A	N/A	1/8"
29	5/8"	1"	1-1/2"	1/32"	3/8"	N/A
29B	N/A	N/A	N/A	N/A	N/A	Superficial
34	1/8"	5/16"	1-1/4"	1/32"	3/16"	N/A
34B	N/A	N/A	1/32"	N/A	N/A	1/64"

**Phase III - Petrography**

Notes:  
1. Depth in inches from either end of the wall.  
2. Severe, moderate and mild refers to the petrographer's subjective assessment of the concentrations of microfractures.  
3. Severe, moderate and mild refers to how much of the cementitious paste exhibited characteristics of unusual sub-round, subhazy and flakiness, etc., and the presence of inclusions of ice crystals.  
4. Occasionally observed up to 1/2".



- ### Conclusions for Residential Walls
- ◆ Concrete temperature not ambient temperature.
  - ◆ Hydration does not stop at 40°F...strength gain continues well below freezing.
  - ◆ Maturity prediction can be used to accurately track in-place strengths.
  - ◆ 500 psi early strength before freezing is reasonable and can be readily achieved.
  - ◆ Current restrictive codes should be relaxed through new techniques and professional practice.
  - ◆ Codes should accommodate better quality control and maturity testing.

- ### Conclusions for Residential Contractors
- ◆ No single mix answer.
  - ◆ Selection of a few mix designs supported by maturity testing to confirm local performance.
  - ◆ Pour earlier in the day – solar gain on concrete mass
  - ◆ Type III cements over Type I for performance
  - ◆ Economical strength gain from use of calcium chloride.
  - ◆ Slower strength gain in cold weather – use caution when removing support.

