

Live Load Testing and Long Term Monitoring of the Varina-Enon Bridge

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Fall 2015 ACI Convention

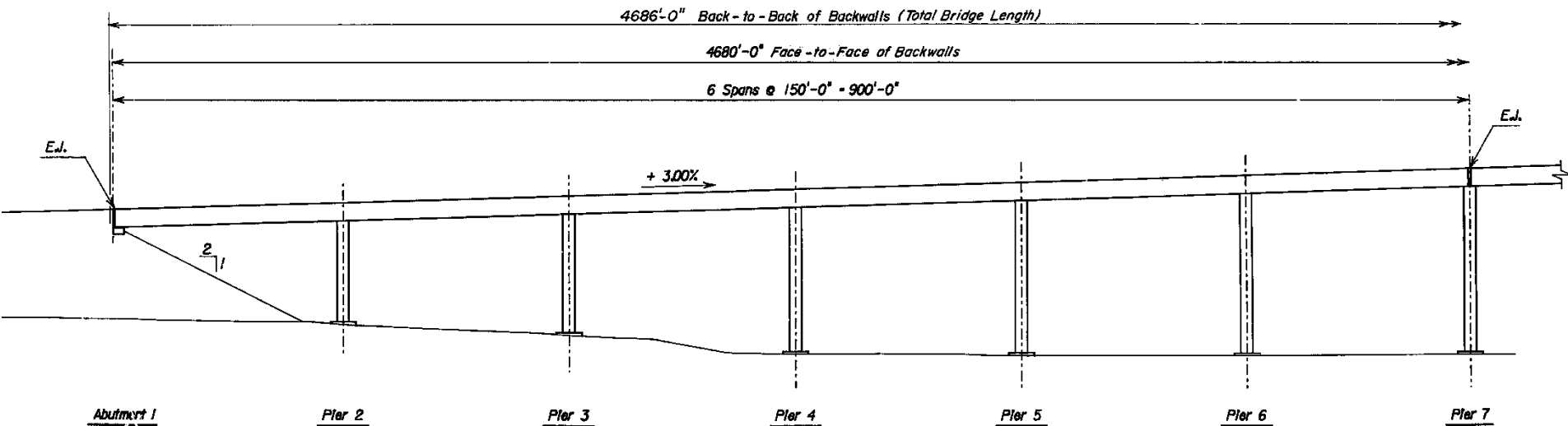
Outline

- Varina-Enon Bridge and observed distress
- Live load test results and comparison to simple models
- Long term monitoring system and one year of data



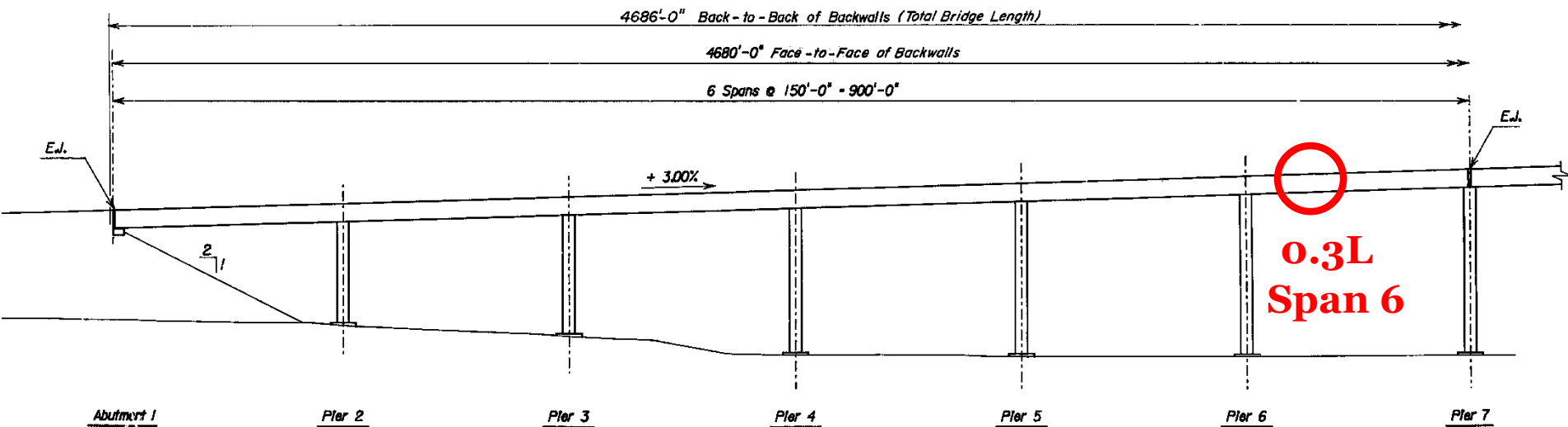
Designed by Figg and Mueller
Designed in 1985
Opened to traffic in 1990

Varina-Enon Approach Structure



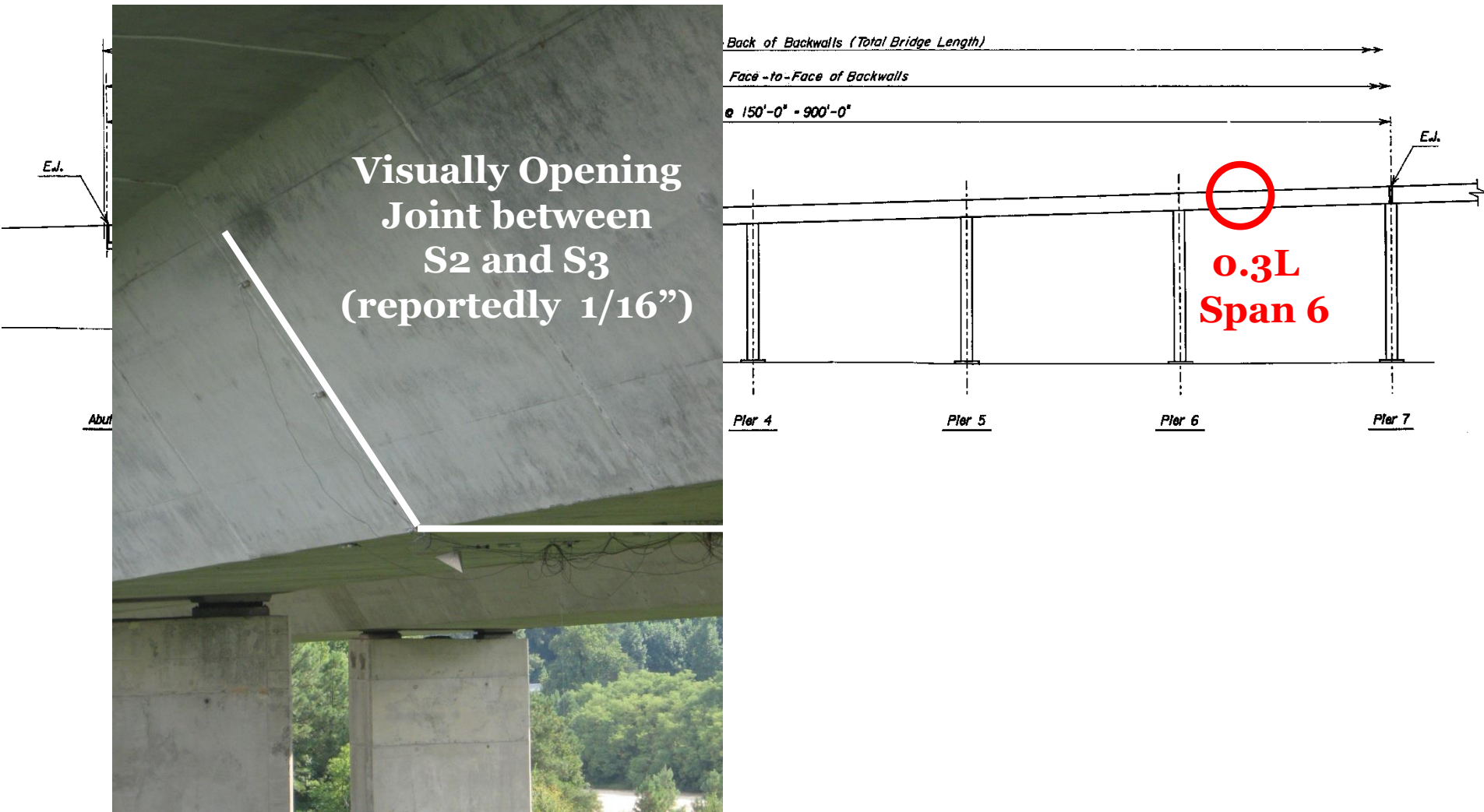
- Northbound, Southern-most Approach Structure Unit
 - Six – 150 ft Spans
 - Externally Post-Tensioned
 - Distress noted by inspectors Summer 2012

Varina-Enon Approach Structure

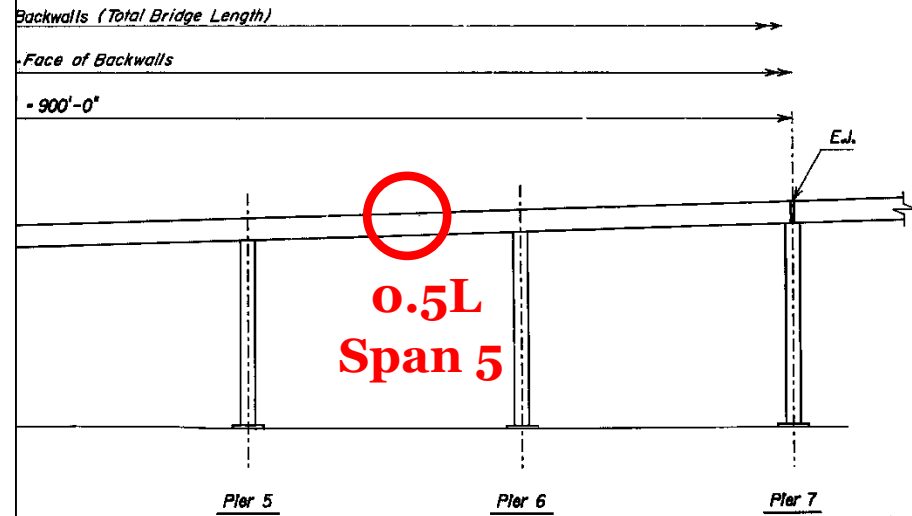
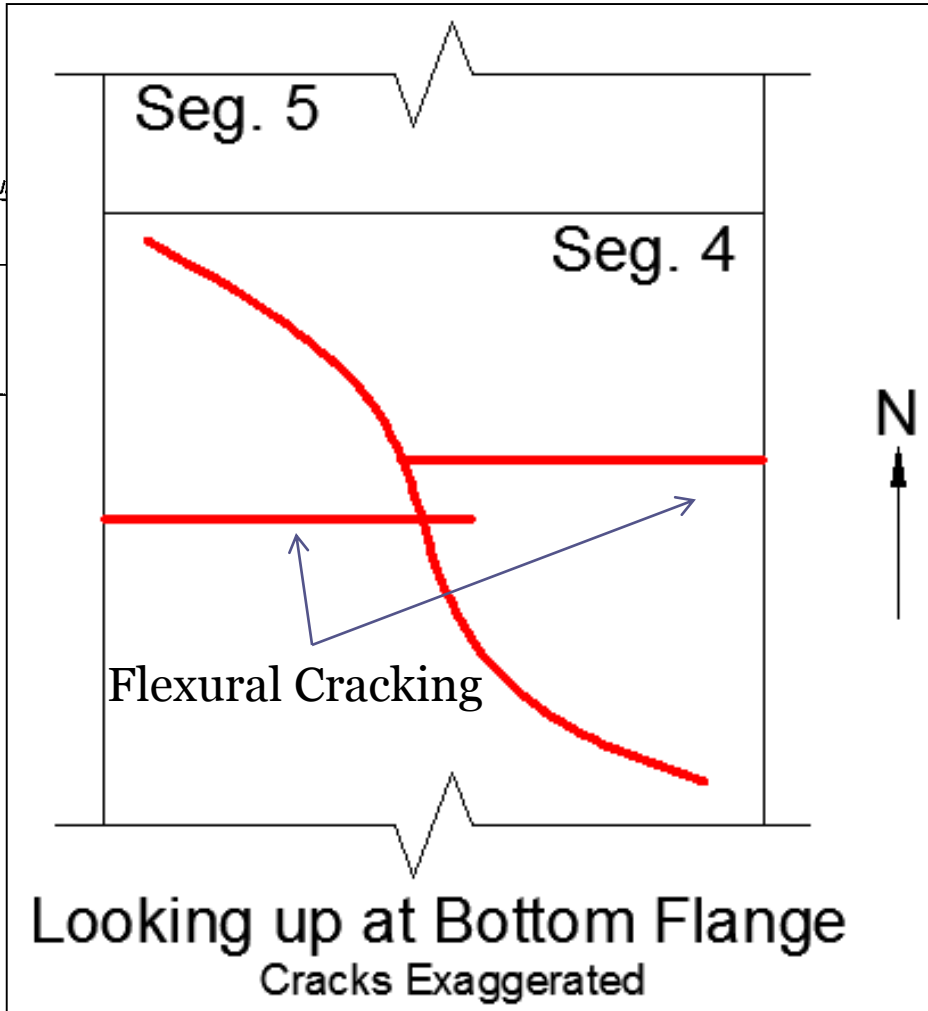


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Varina-Enon Approach Structure

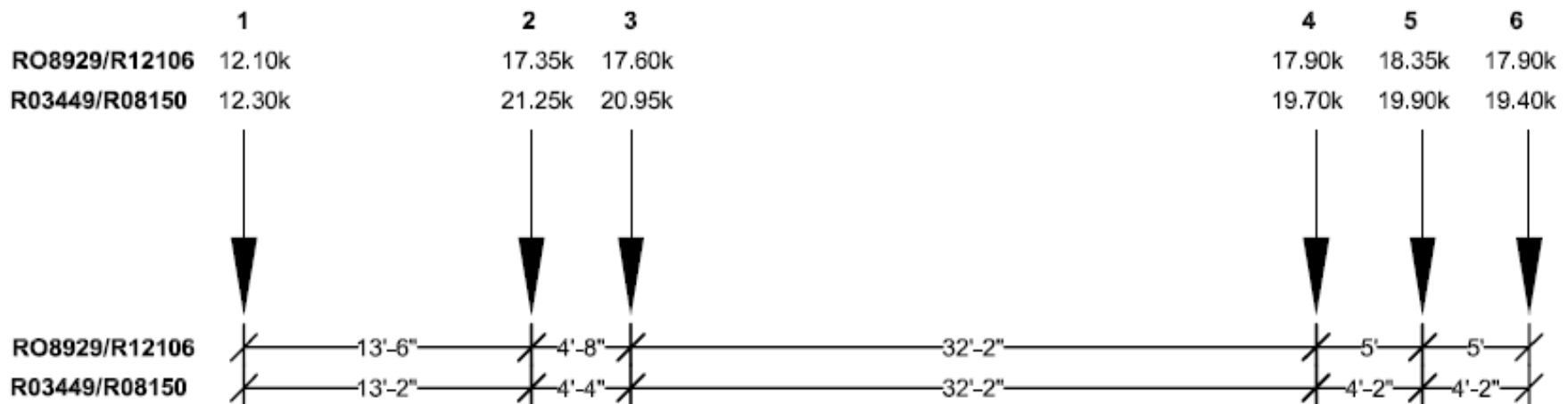


Varina-Enon Approach Structure

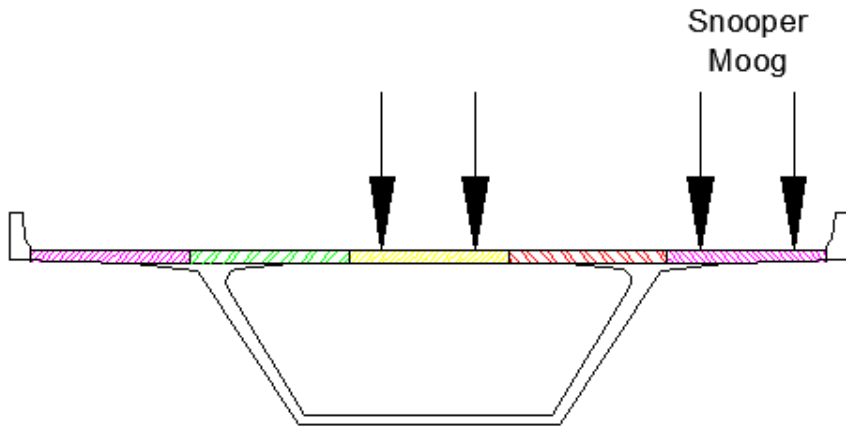


Live Load Test Performed August 2012

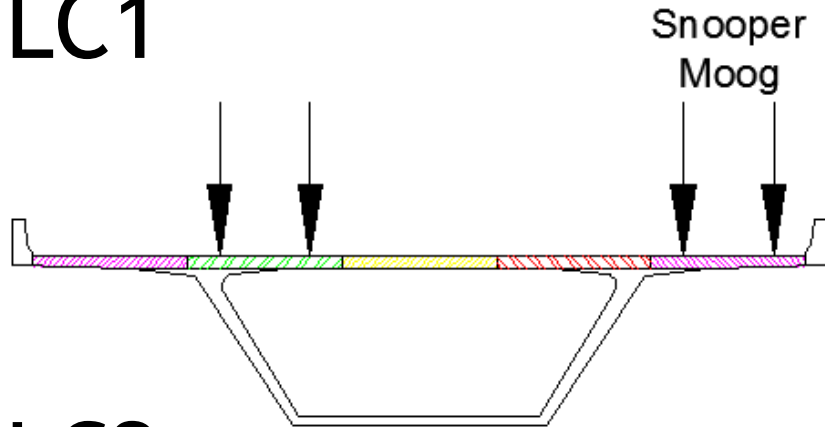
- Two Low Boy Tractor-Trailers
 - R12106 – 101.2 kips
 - R08150 – 113.5 kips



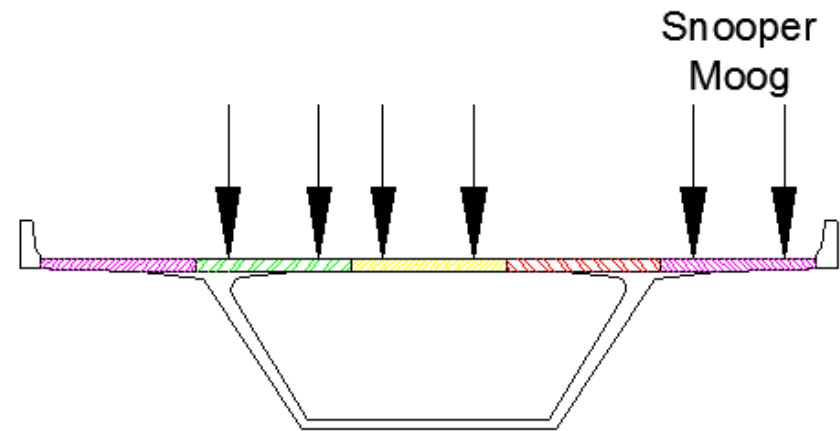
Three Truck Crossing Paths



LC1

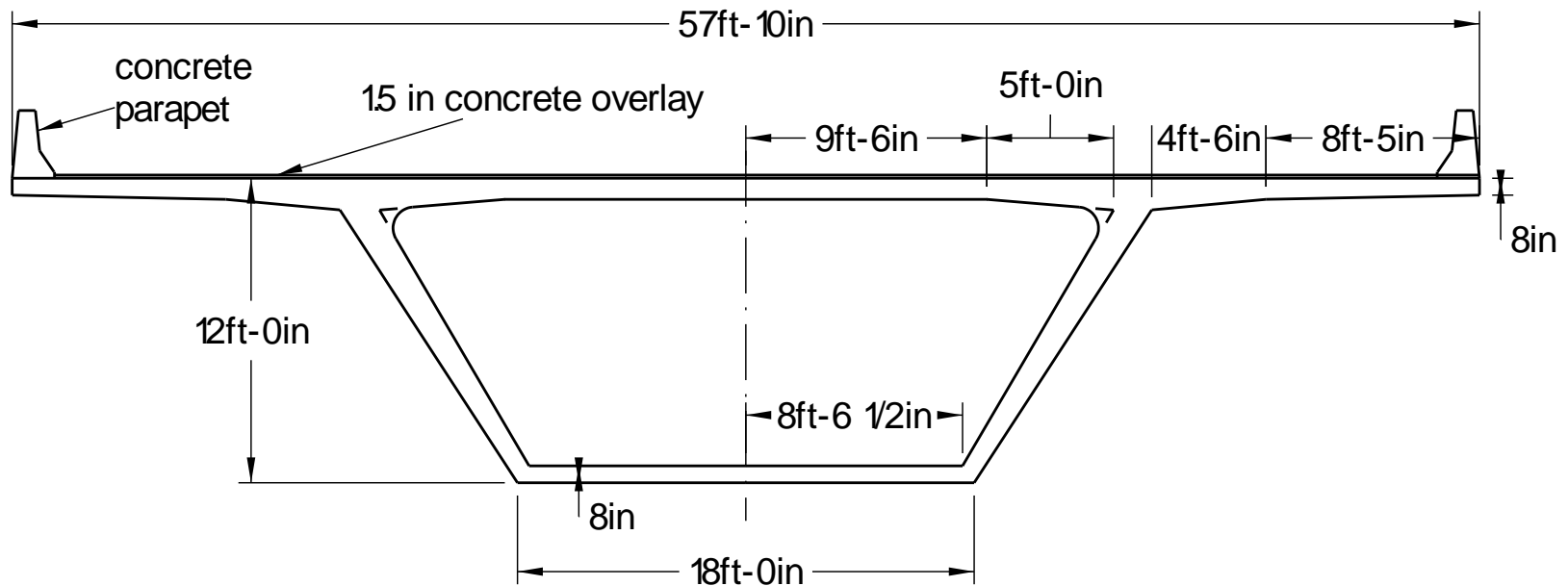


LC2



LC3

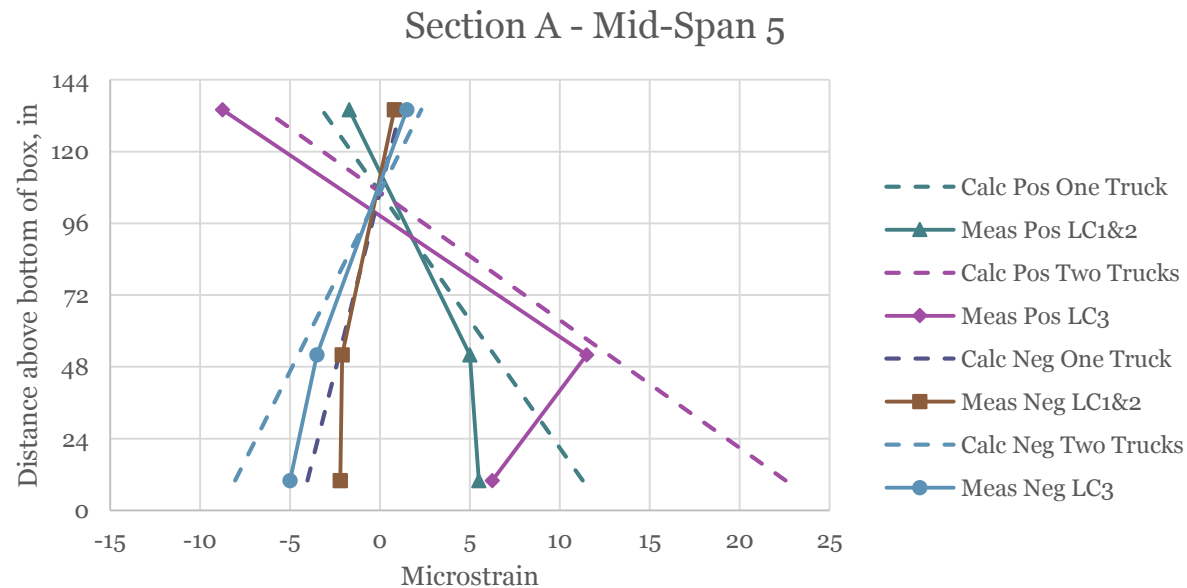
Simplified Beam Line Analysis



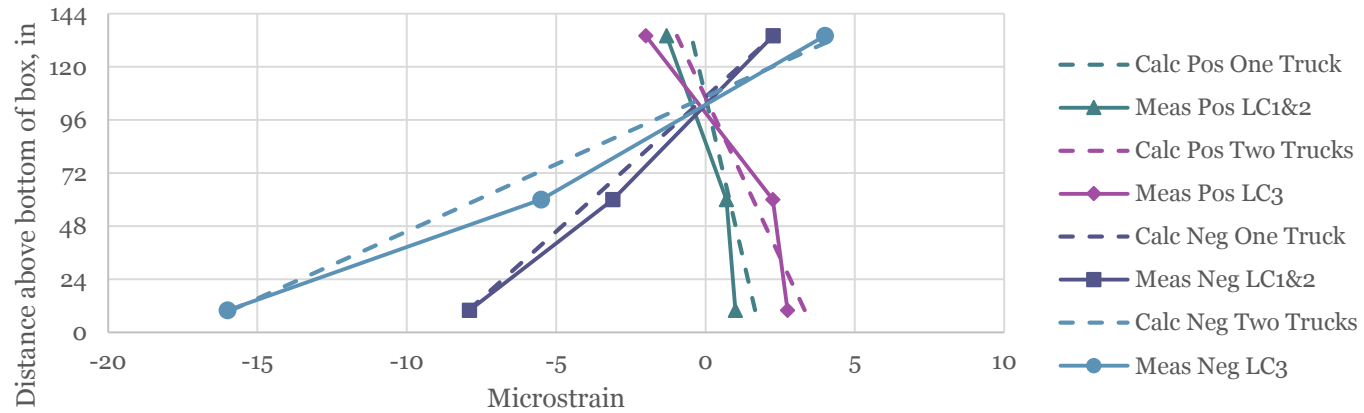
	Concrete Box Only	Concrete Box plus Overlay, Barrier, Rebar and Tendons
Area	12630 in ²	14730 in ²
Moment of Inertia	33,853,000 in ⁴	39,771,000 in ⁴
Centroid (from bottom)	101.9 in	106.4 in

Comparisons to Tests

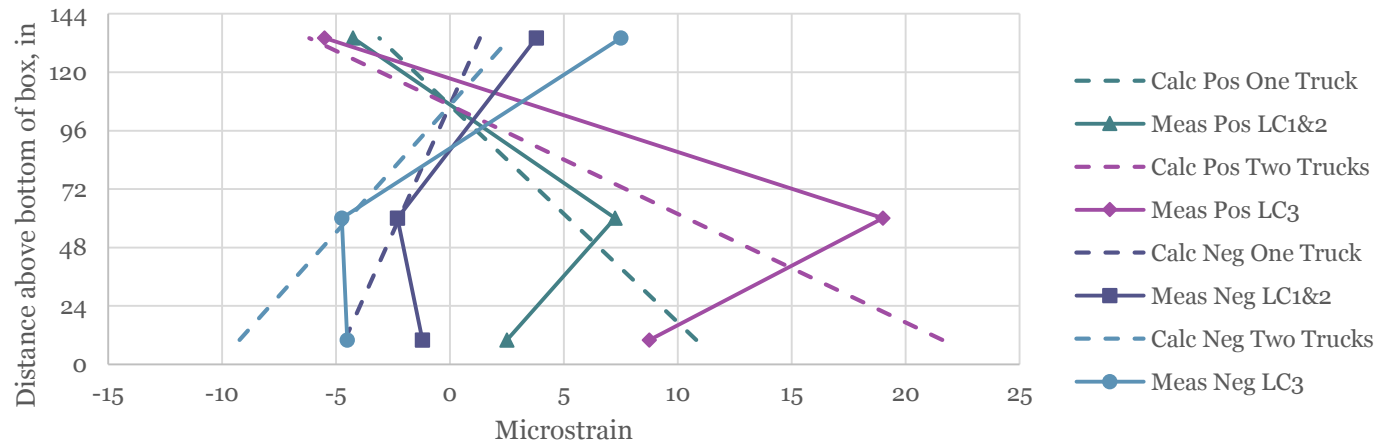
	LC1		LC2		LC3	
	LC1-1	LC1-2	LC2-1	LC2-2	LC3-1	LC3-2
Measured Deflection, in	0.030	0.030	0.035	0.034	0.062	0.062
Calculated Deflection, in	0.031		0.031		0.062	



Section C - Near Support Span 5



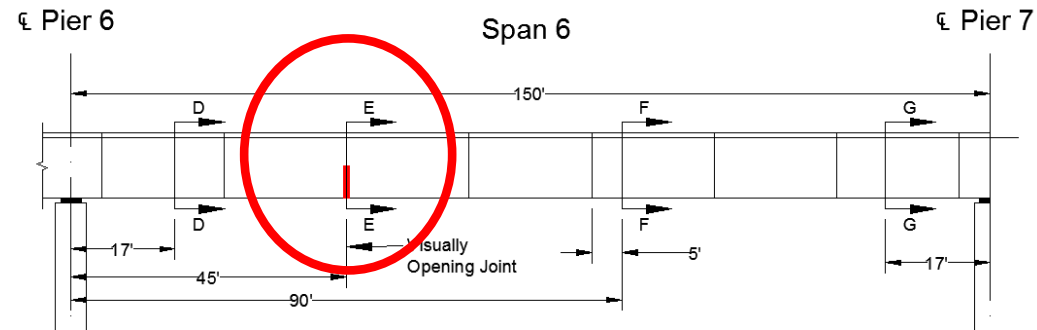
Section E - Opening Joint Span 6



Recap

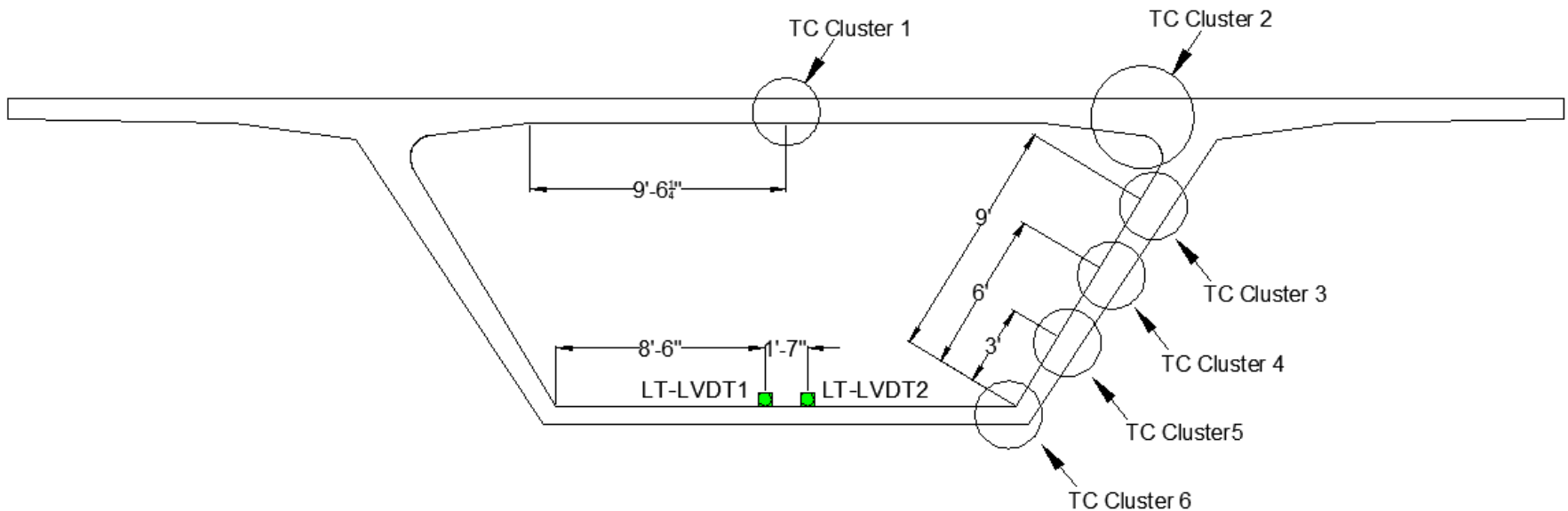
- Visual estimate of joint opening prior to load test was 1/16 in. under traffic
 - Opening was observed on June 29, 2012
 - High temperature according to NOAA at the airport was 103 degrees F
- Joint opening measured during load tests was 1/64 in. under load
 - Test was performed on August 21, 2012
 - Heaviest test vehicle 113.5 kips (rear three axles 60k)
 - Temperature at test time was mid 80s F

Long Term



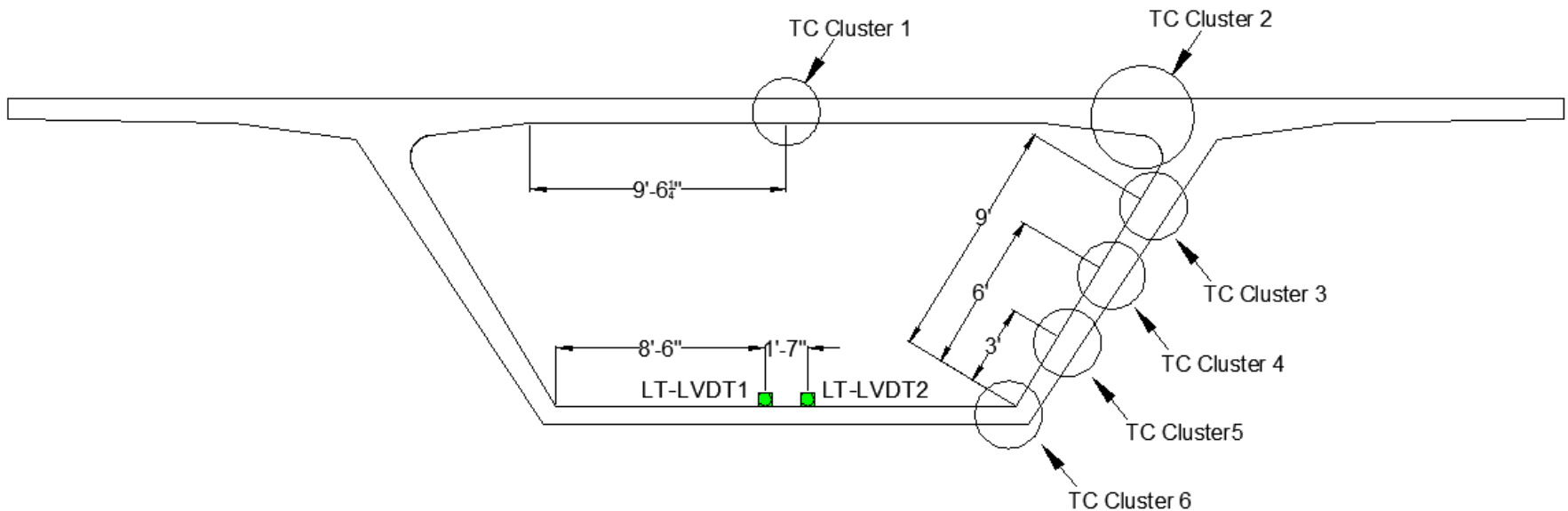
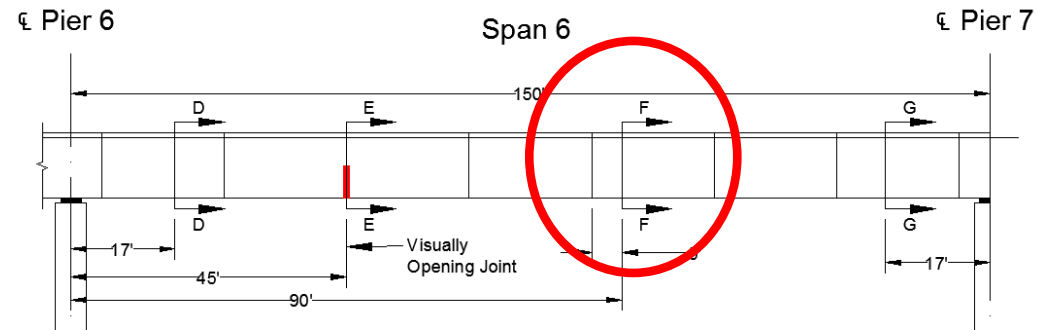
- S2/S3 Joint

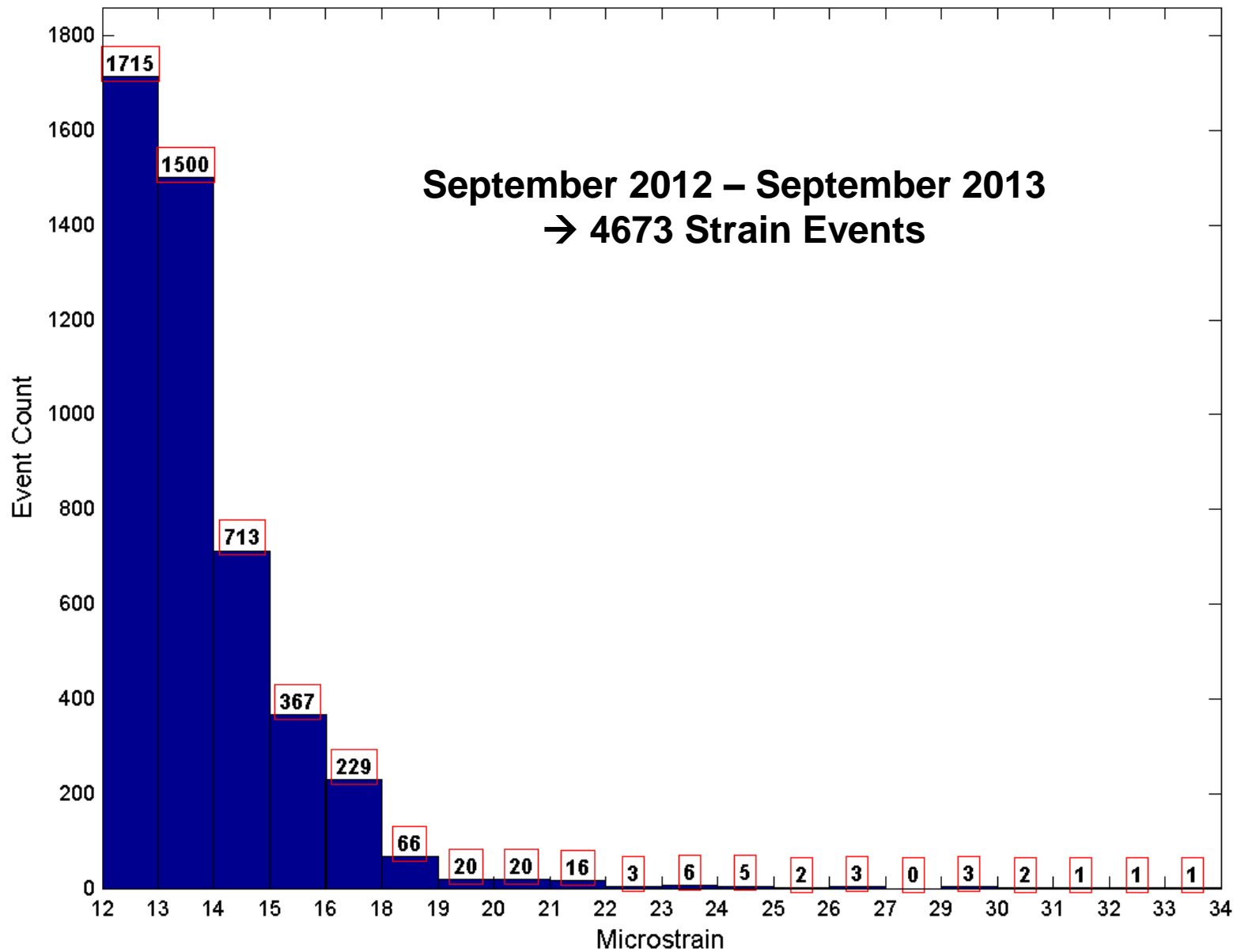
- 2 LVDTs monitoring crack opening
- 1 Strain transducer next to crack
- 24 Thermocouples measuring thermal gradient

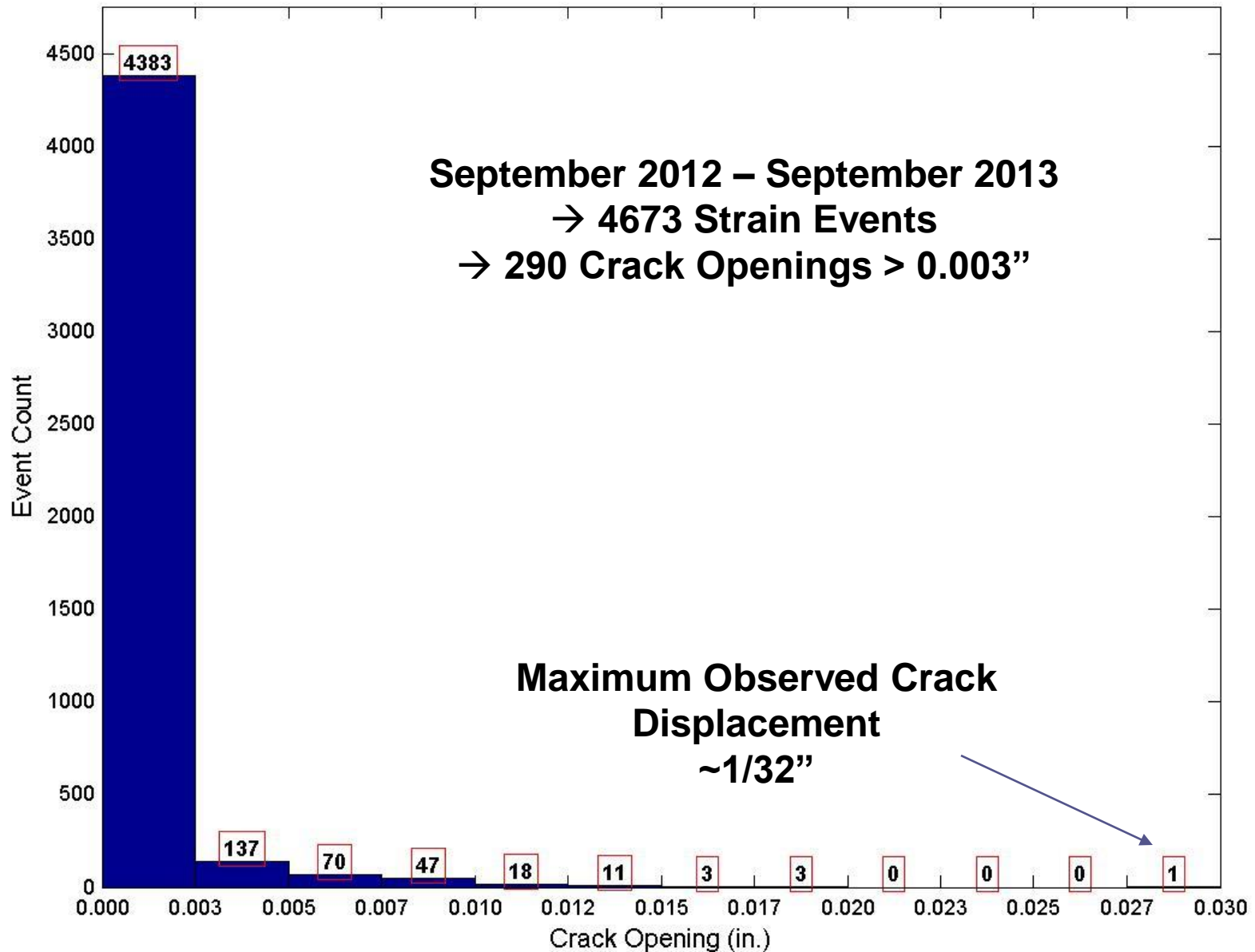


Long Term

- 0.4L
 - Single strain transducer at 0.4L from bridge end
 - Used to trigger events, trigger set at $12 \mu\epsilon$

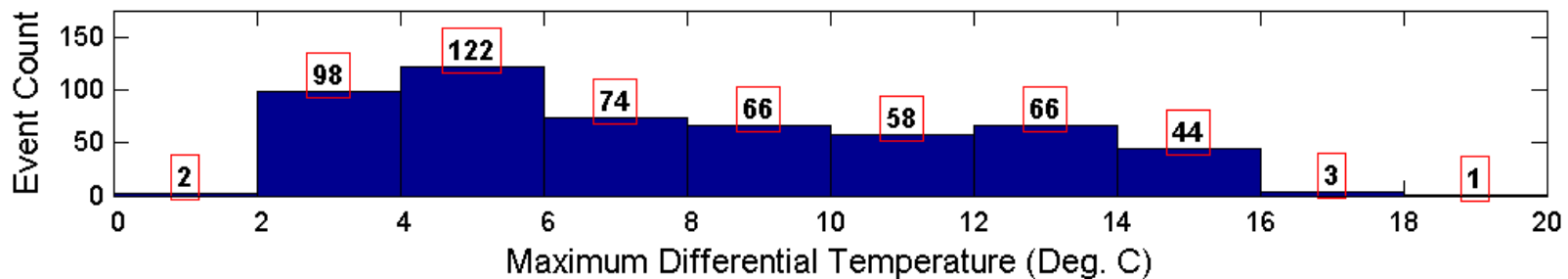
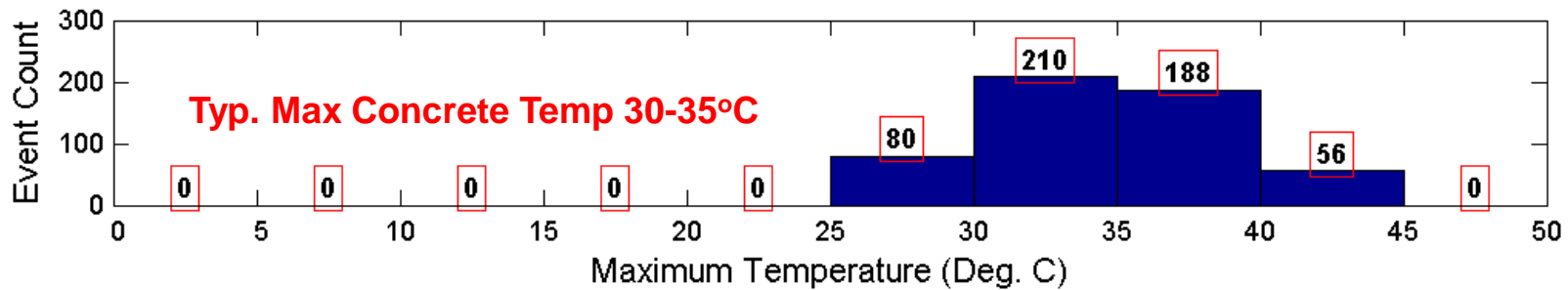
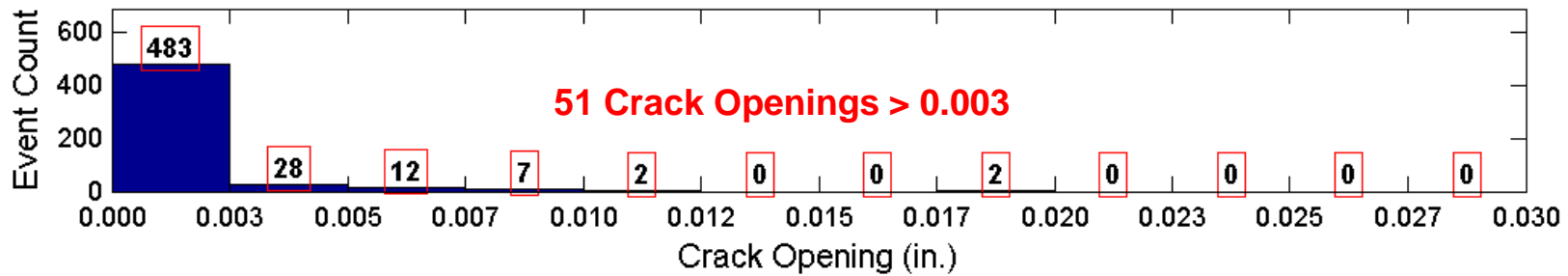
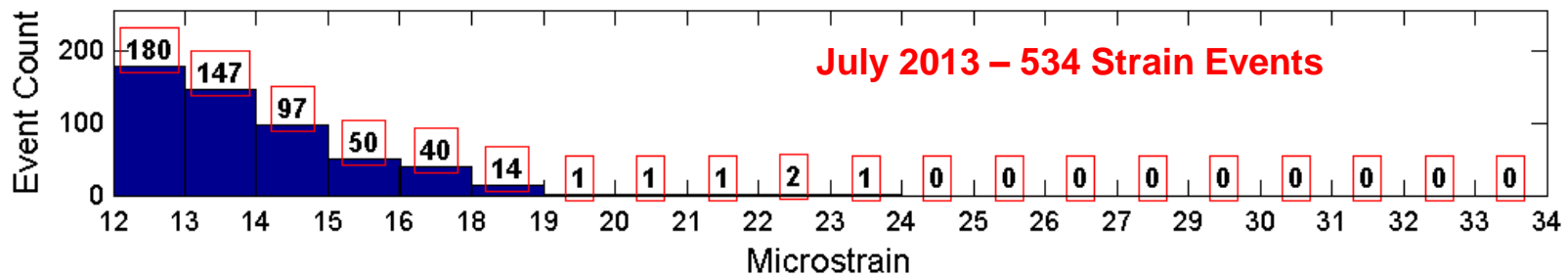


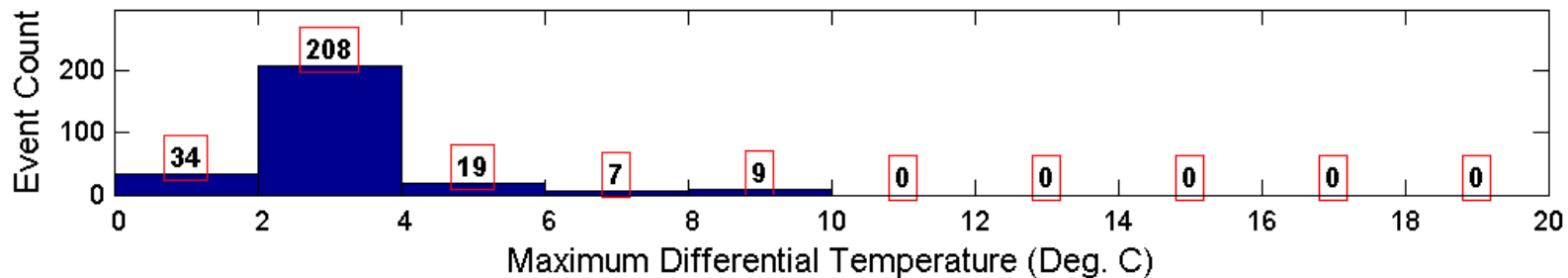
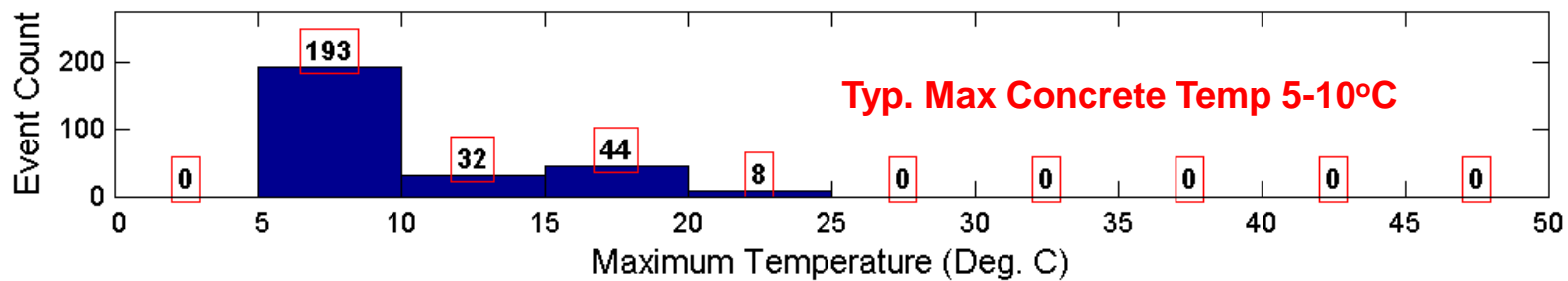
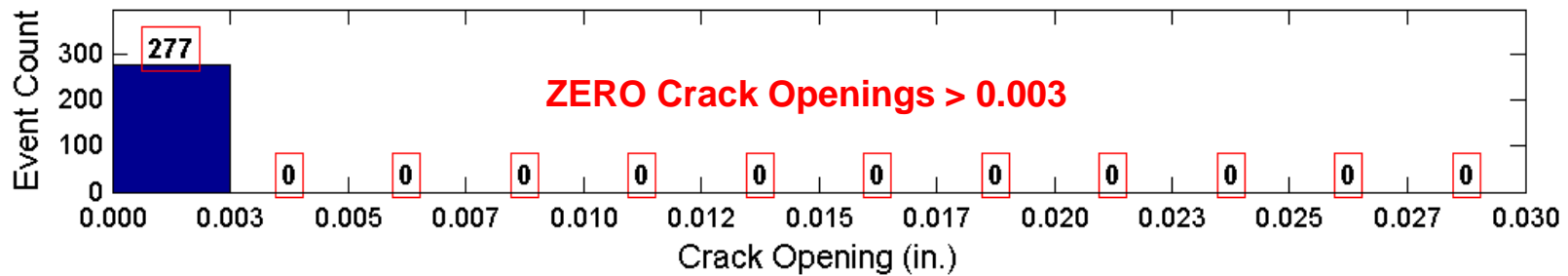
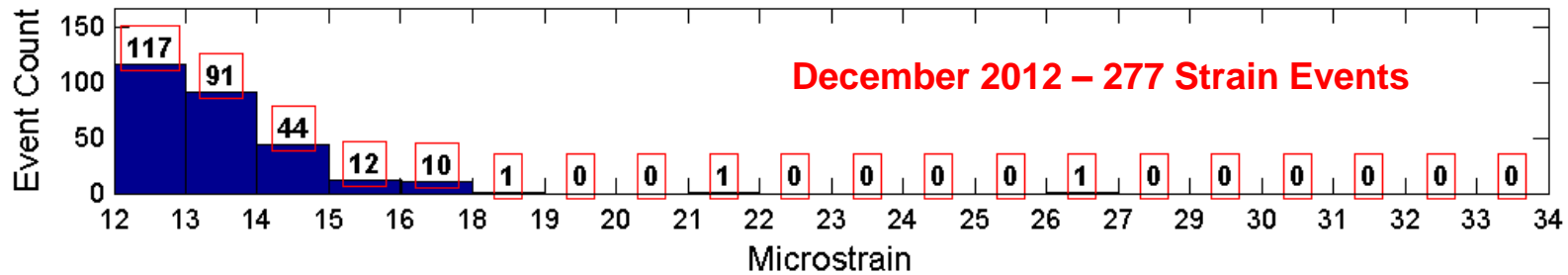




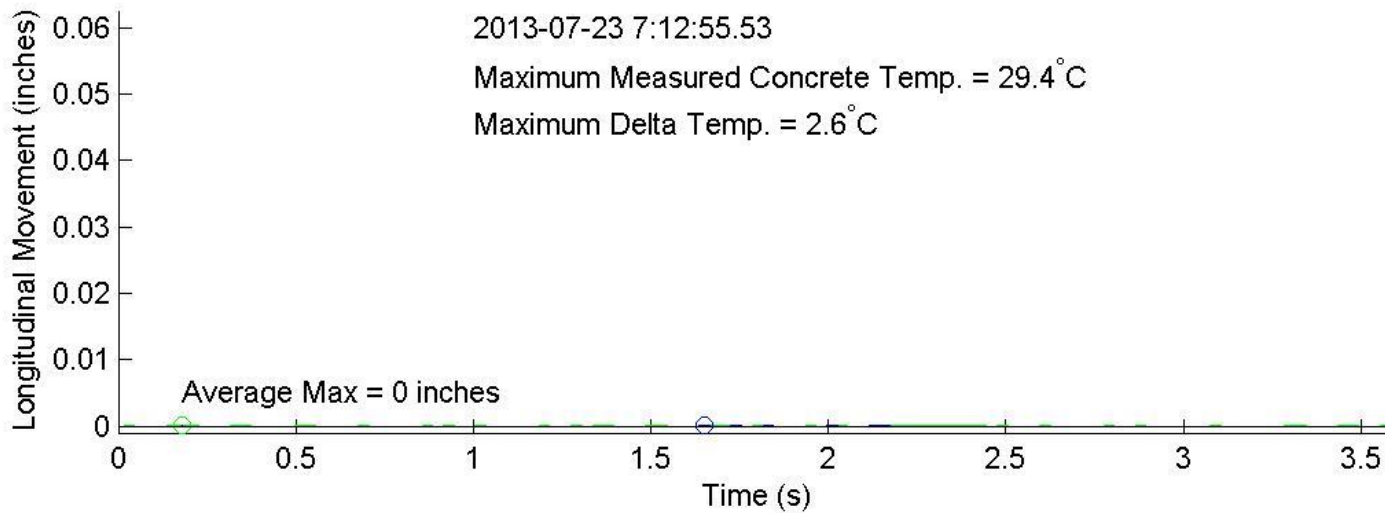
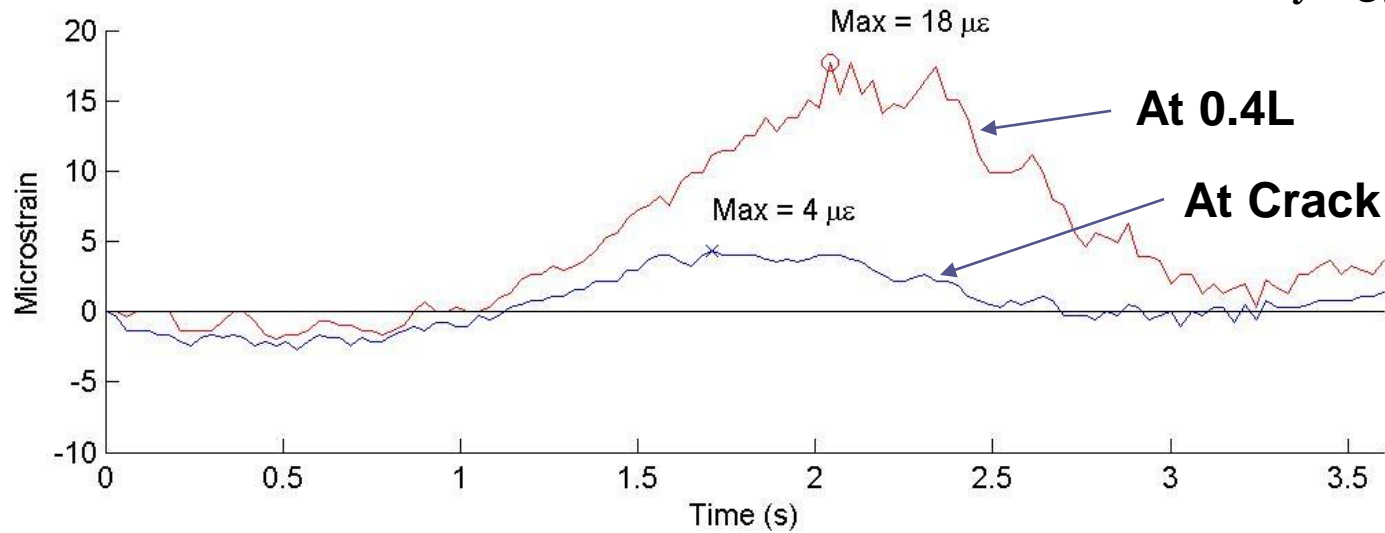
Influence of Temperature

- Positive thermal gradients induce bottom slab tension at critical joint location
- Compare warm month and cold month data to investigate
- AASHTO Design Thermal Gradient is 41°F (23°C) non-linear
- Actual design gradient, with live load, was 9°F (5°C) linear

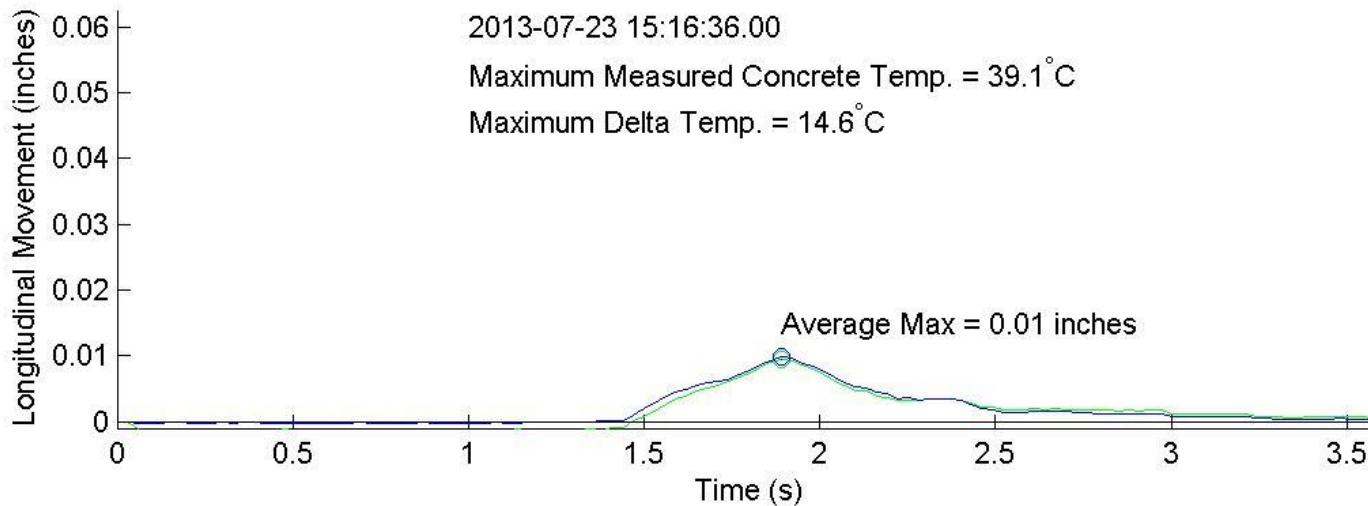
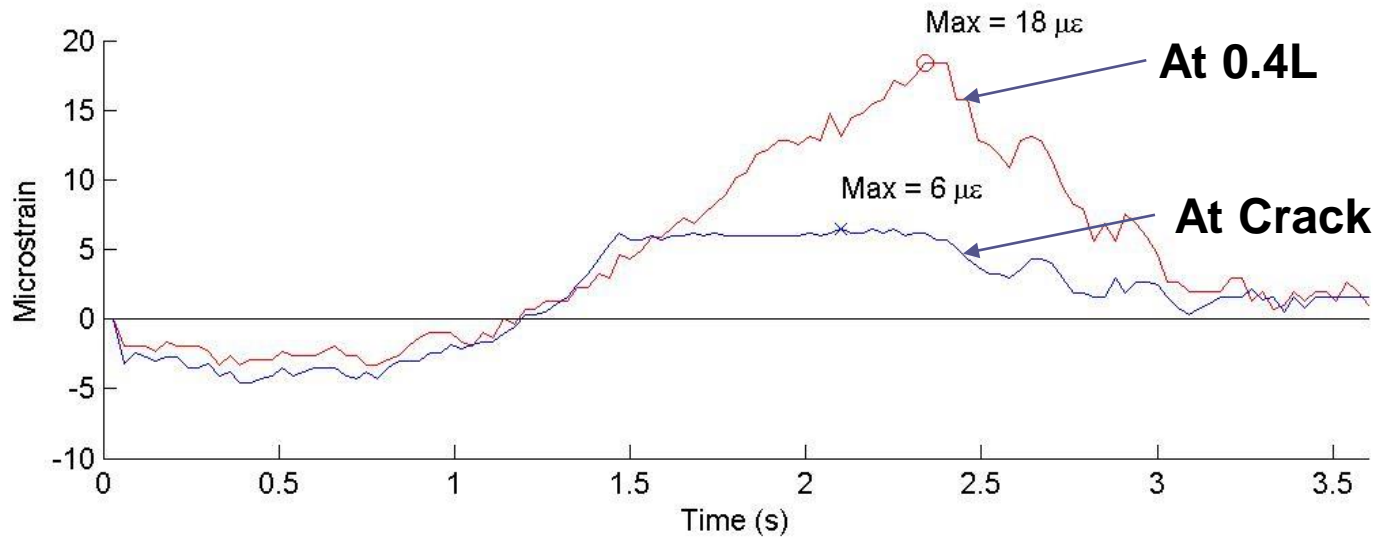


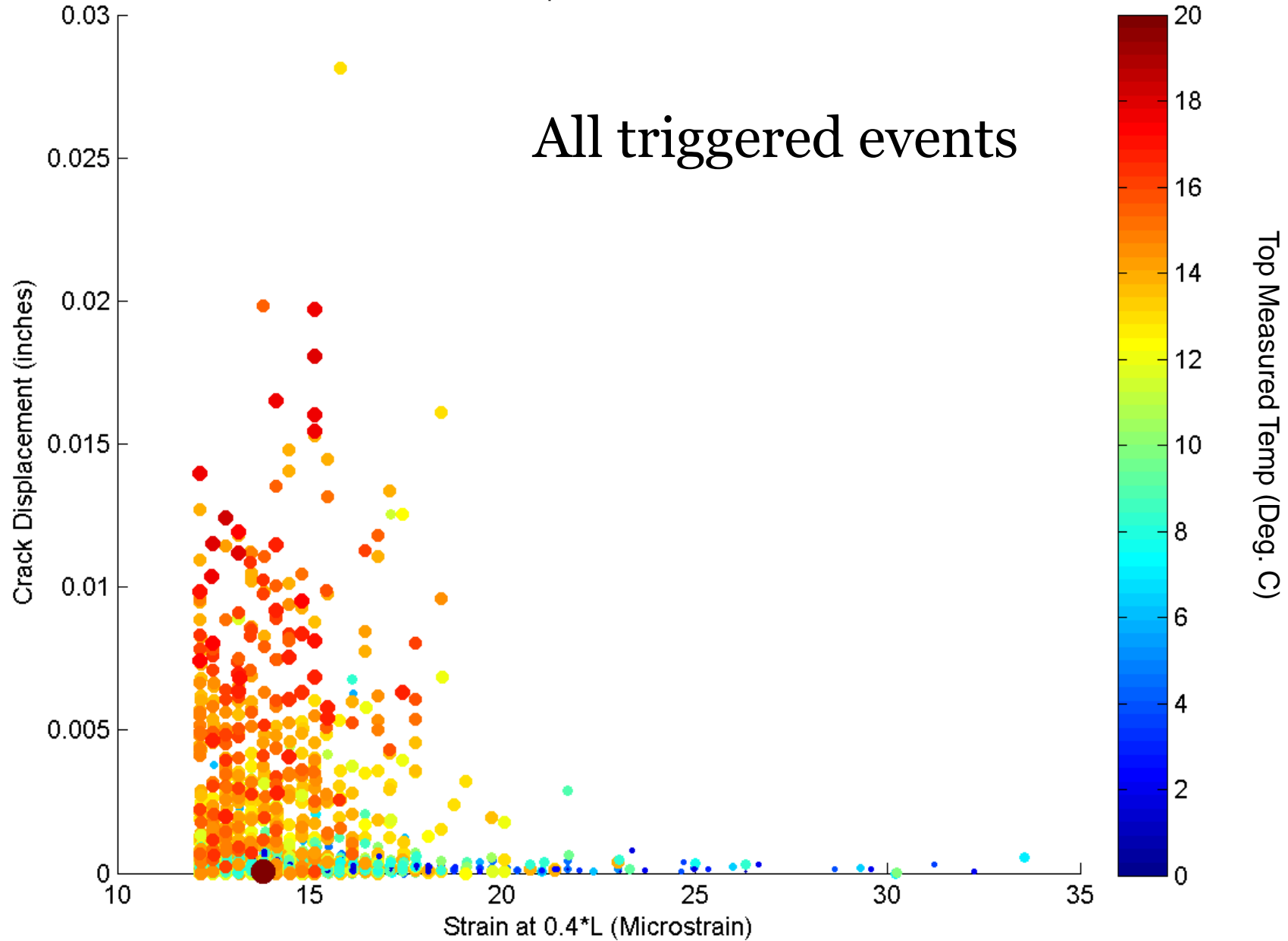


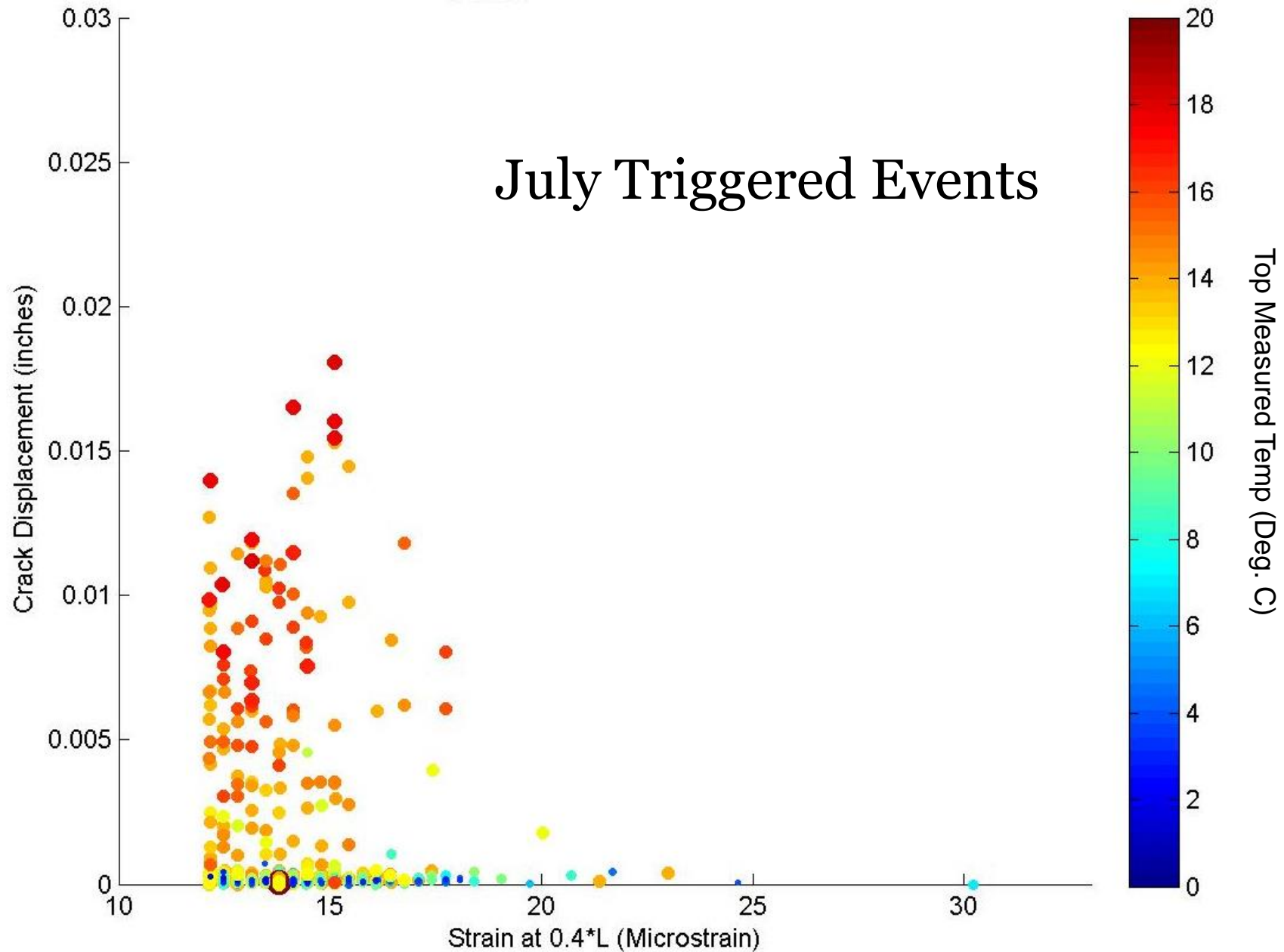
July 23, 7am, 2.6°C diff.

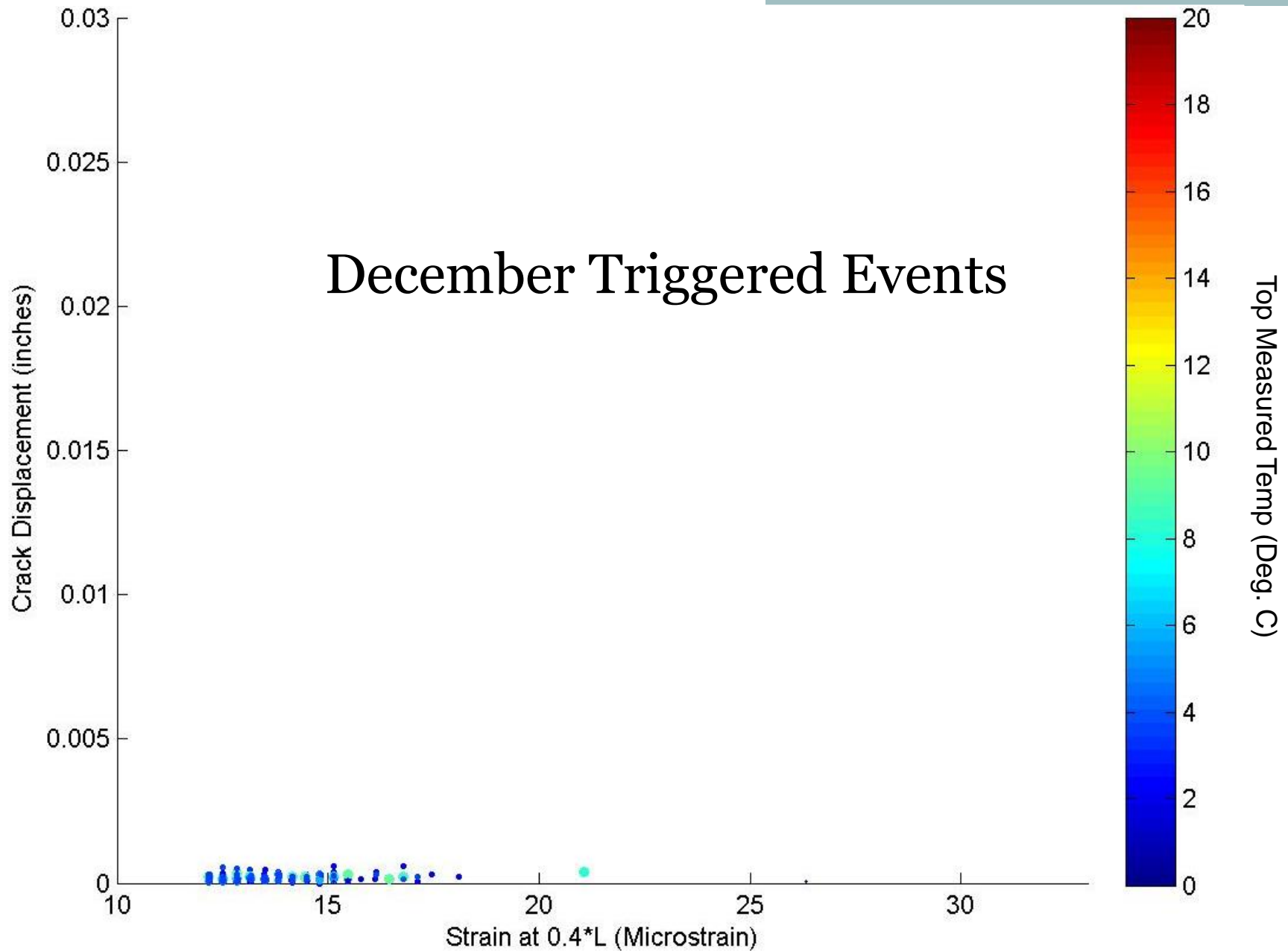


July 23, 3pm, 14.6°C diff.









Simplified Long Term Analysis

Equation

$$M_f = M_{initial}e^{-\phi} + M_{final}(1 - e^{-\phi})$$

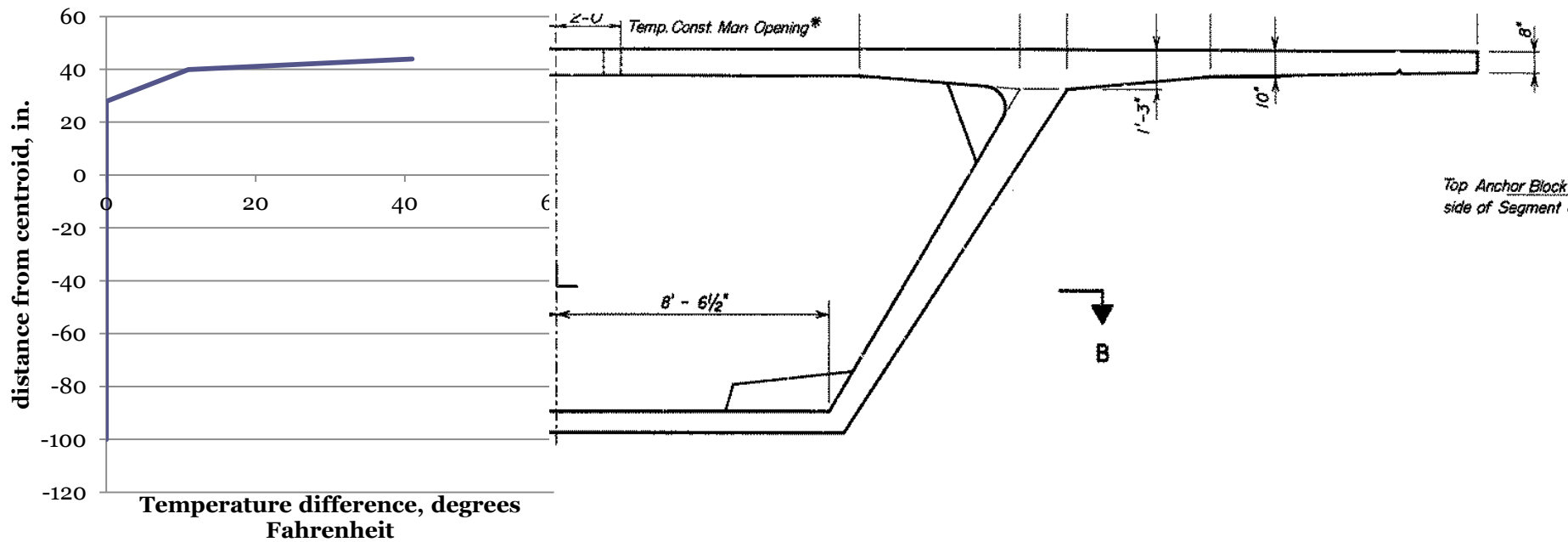
For $\phi = 0$ – no change in moment distribution

For $\phi = \infty$ - Creeps completely to final moment distribution

For $\phi = 2.5$ – Creeps over 90% toward final moment distribution

Analysis done for final system configuration (close enough)

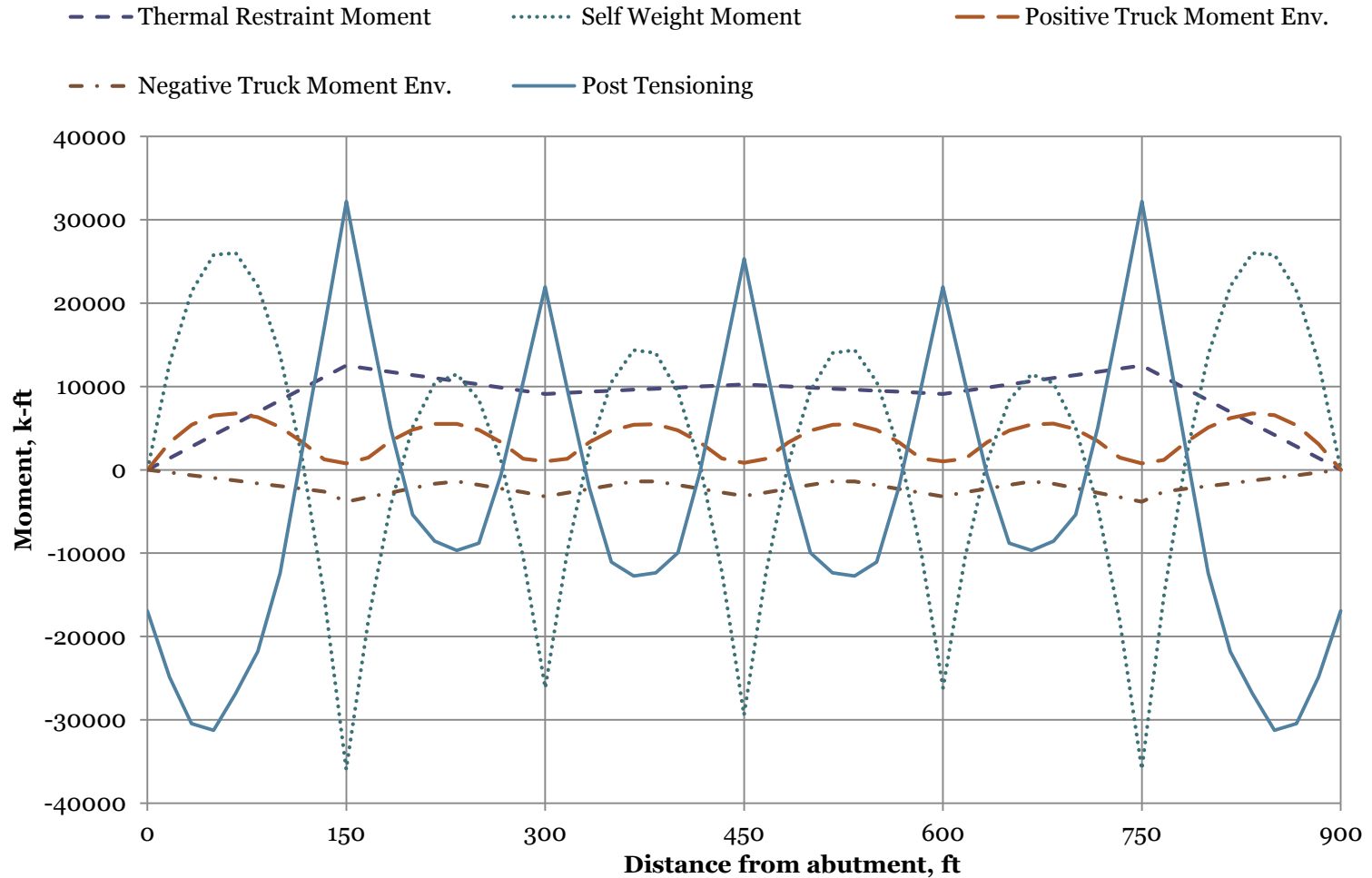
Thermal Gradient



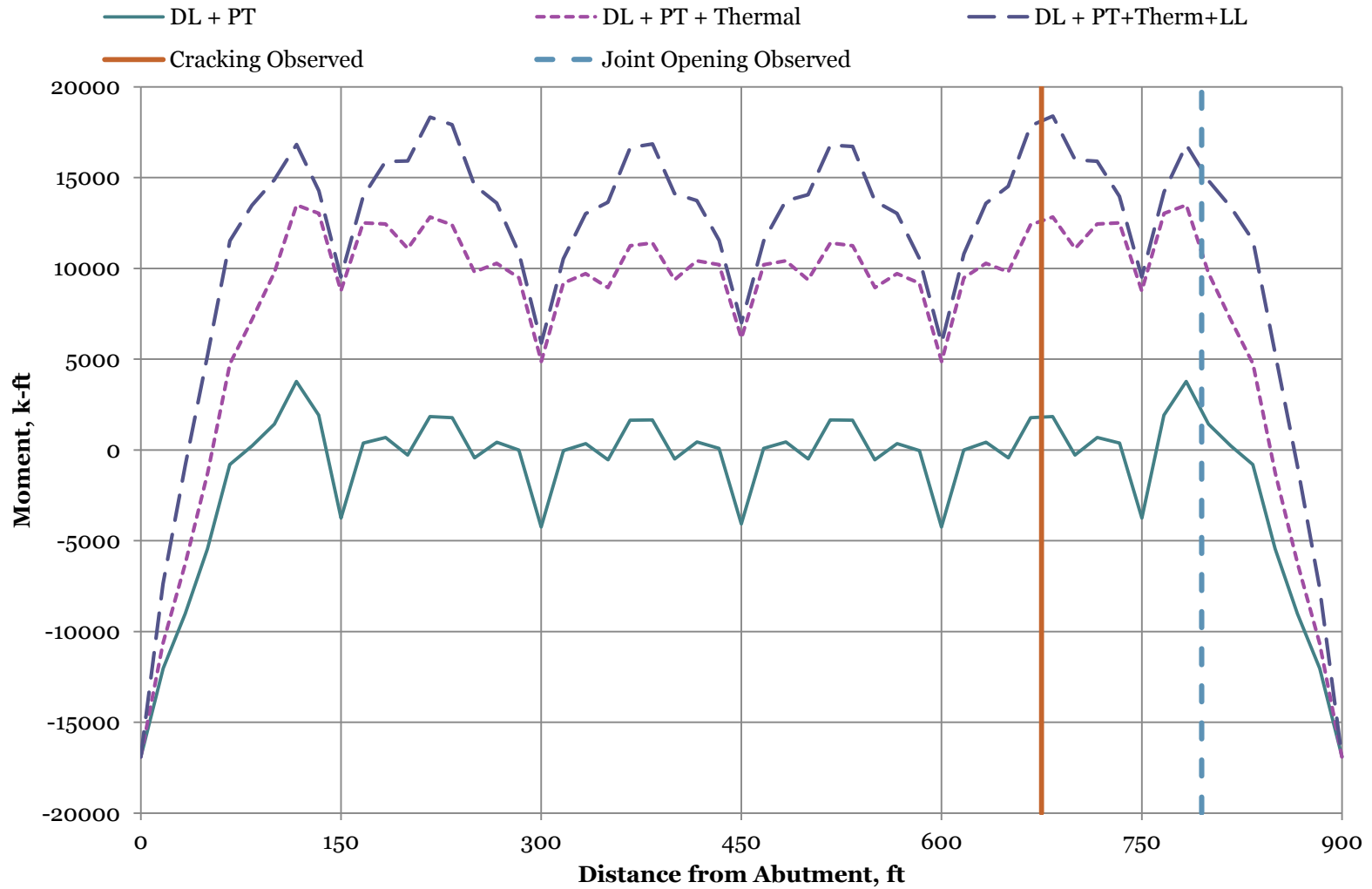
$$M_{thermal} = \int_Y E \alpha T(Y) b(Y) Y dY$$

$$\text{Moment} = 8727 \text{ k-ft}$$

All Moments



Summations



Summary

- Maximum joint movement during load test:
 - 1/64 in.
 - Less than previously observed: 1/16 in.
- Maximum joint movement from ambient traffic over the past year:
 - 1/32 in.
- Thermal gradient is affecting severity and frequency of crack opening
- Monitoring is continuing and other locations will be investigated

Questions?