

Load Test of a Precast Concrete Tub Stadium Riser and Bond Testing of #20 Bar



Presented by

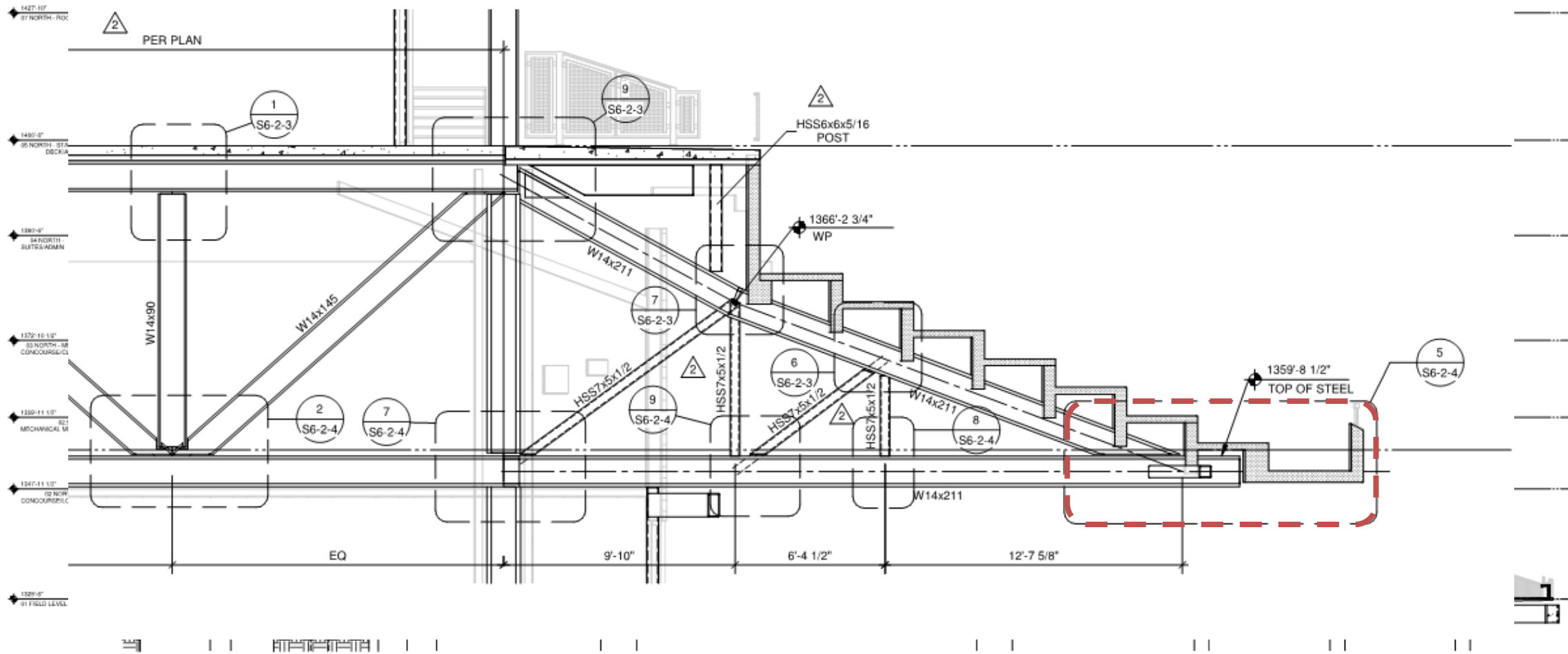
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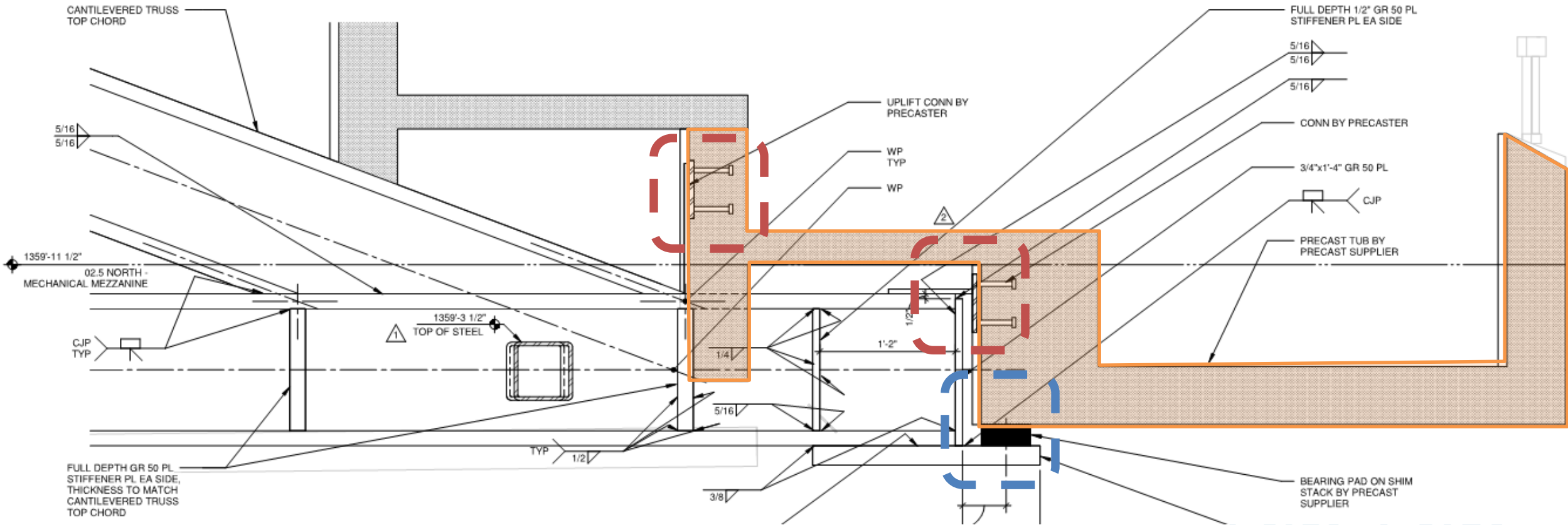
The Project

- Expansion at a collegiate football stadium
- Mezzanine club level in endzone
 - Cantilevering precast concrete tub riser
 - Supported by steel truss





Concrete Tub Riser

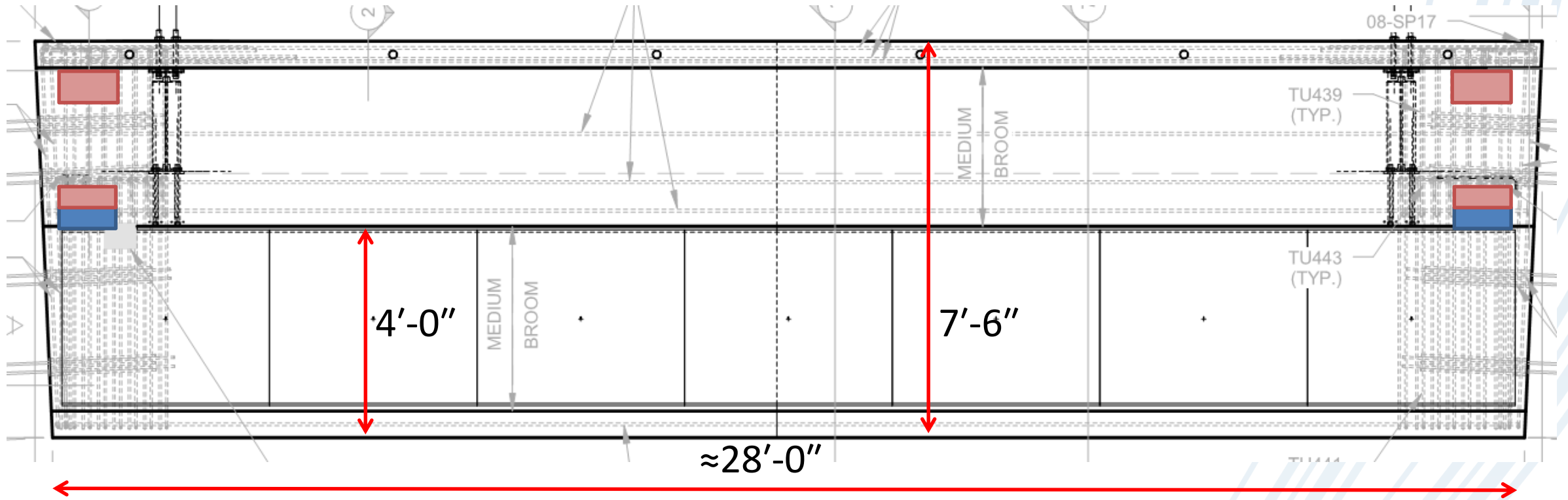


Plan View

- Which direction controls?

$$\frac{l_{long}}{d} = \frac{28ft}{2ft} = 14$$

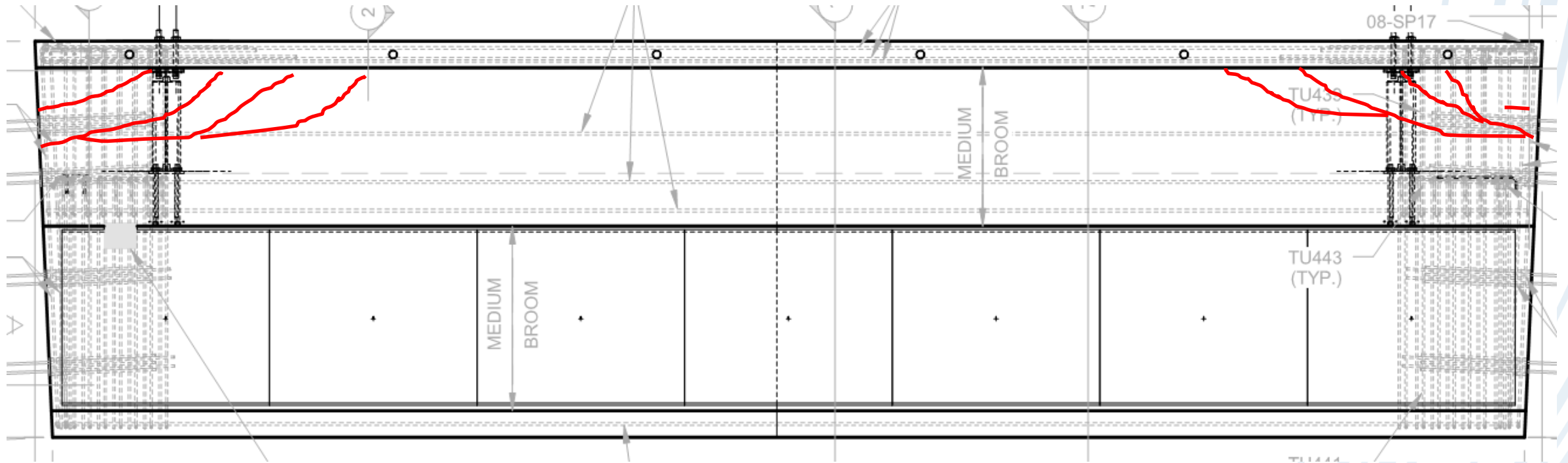
$$\frac{l_{tran}}{d} = \frac{2 \times 48in}{5.5in} = 17$$



Distress



Cracking in non-cantilever portion



- Consistent for bottom-most mezzanine concrete tub risers

Retrofit and Load Test

- Bending response
 - Longitudinal
 - Transverse cantilever
 - Torsional

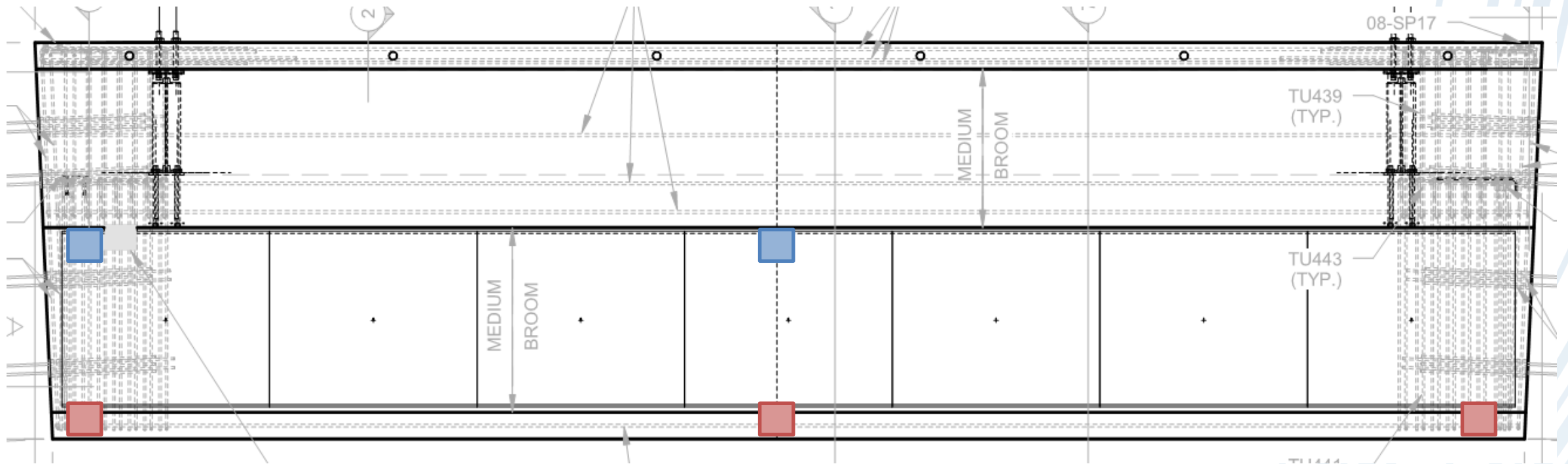


Load Test per ACI 318-11

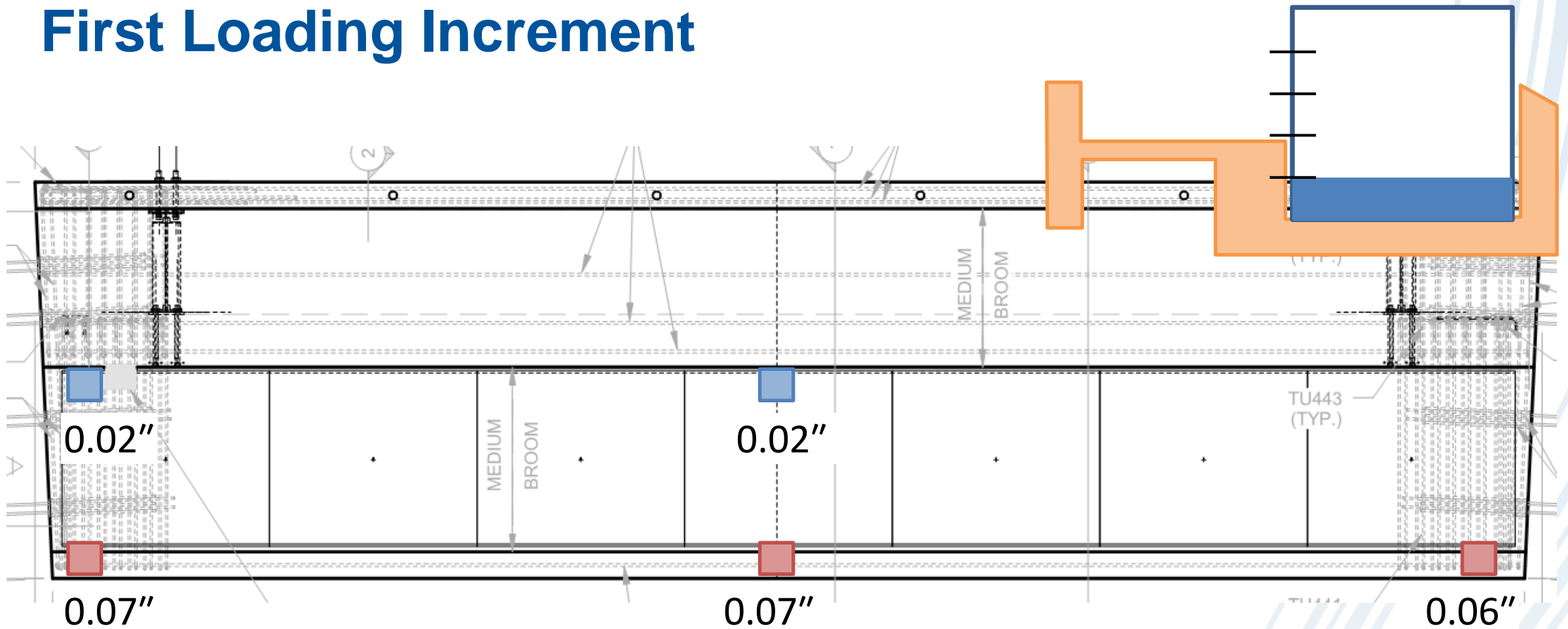
- Load cantilever portion
- Design live load = 100 psf
- Total test load = 150 psf
- Use industrial totes for loading
 - 30 inches of water
- Load
 - Four equal increments with 24-hr hold
 - Measure deflection with string potentiometer
 - Measure crack width



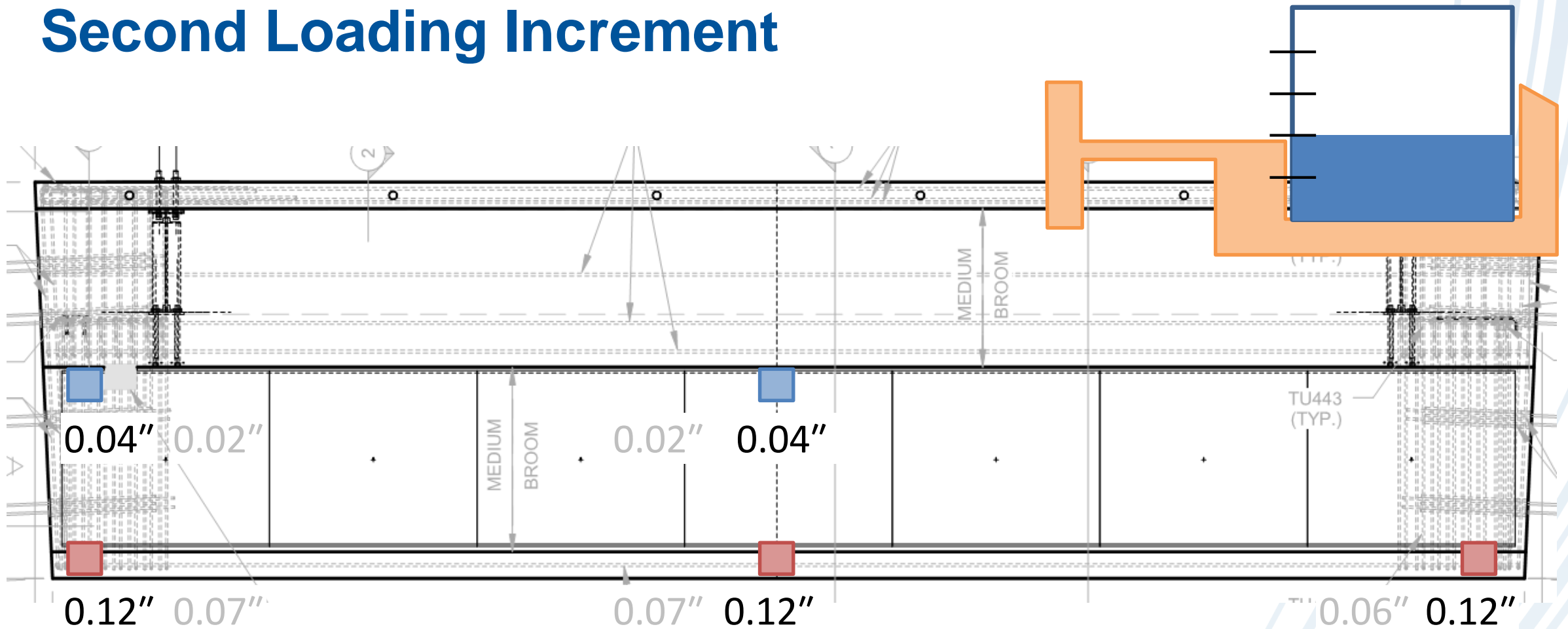
Instrumentation Locations



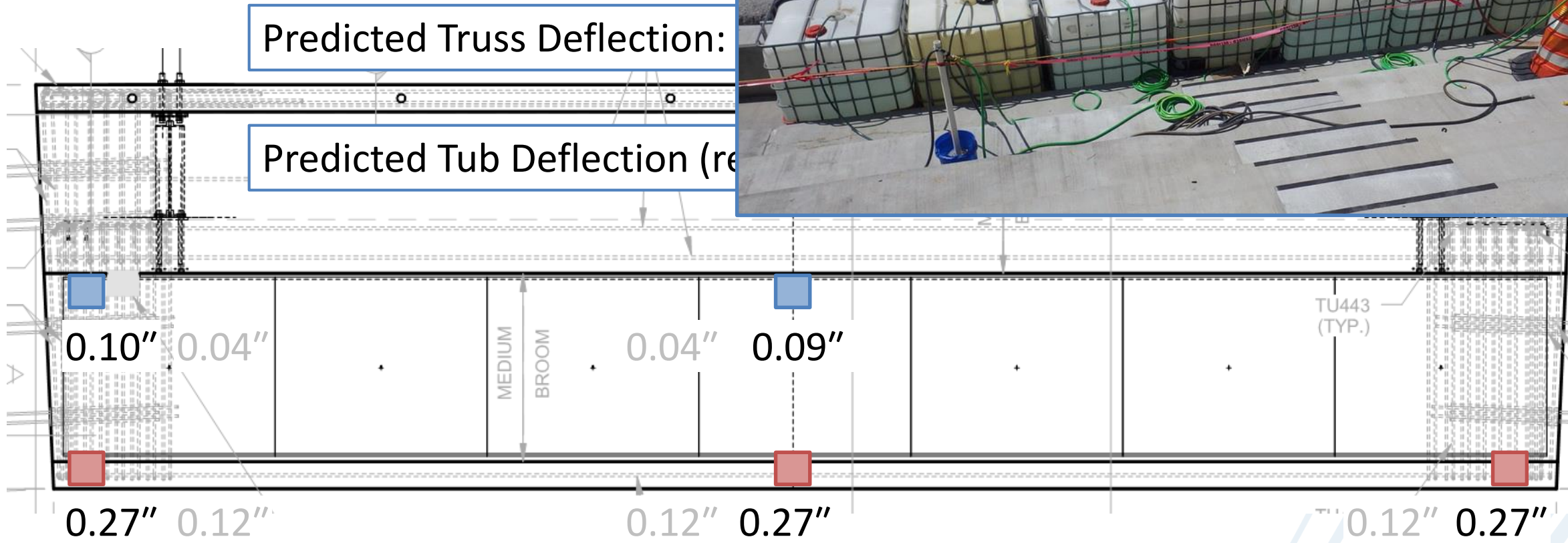
First Loading Increment



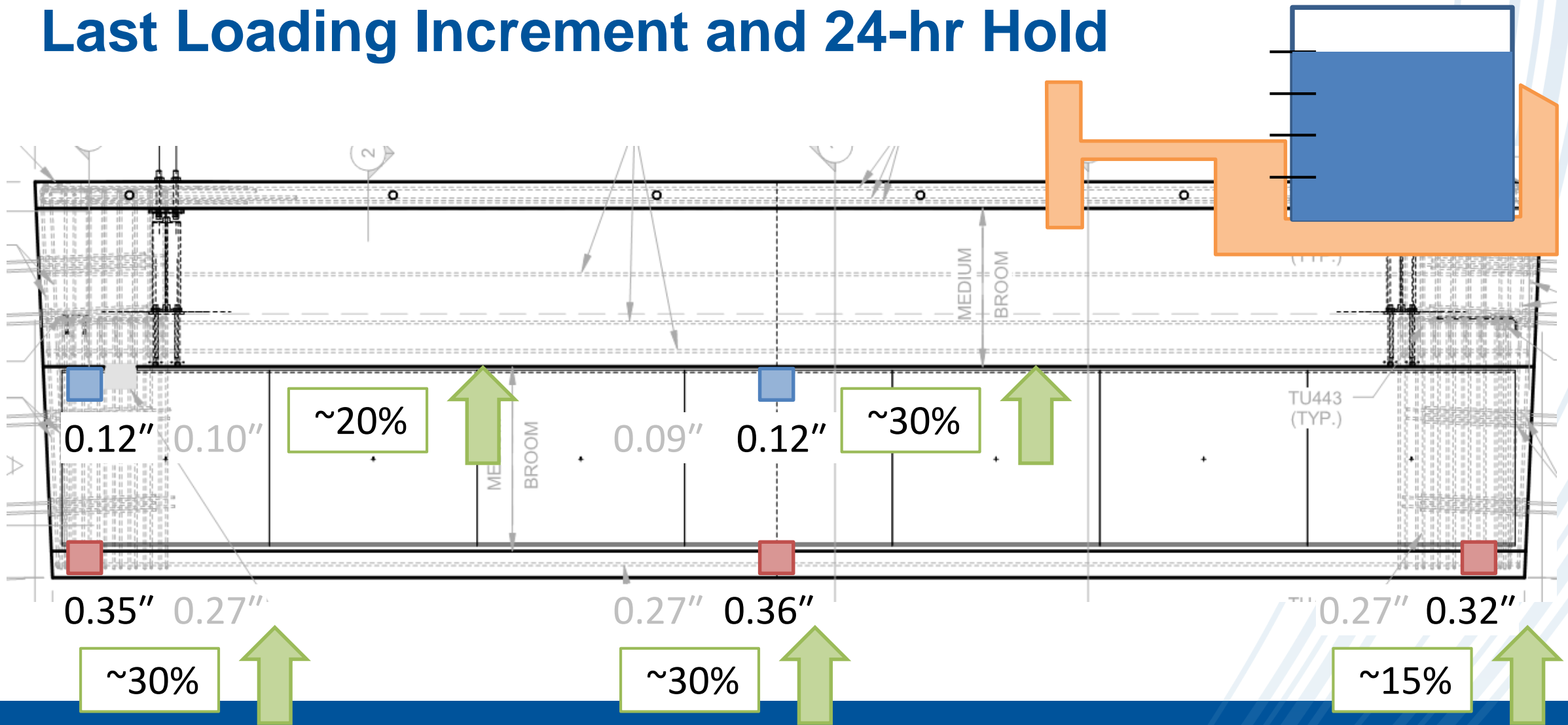
Second Loading Increment



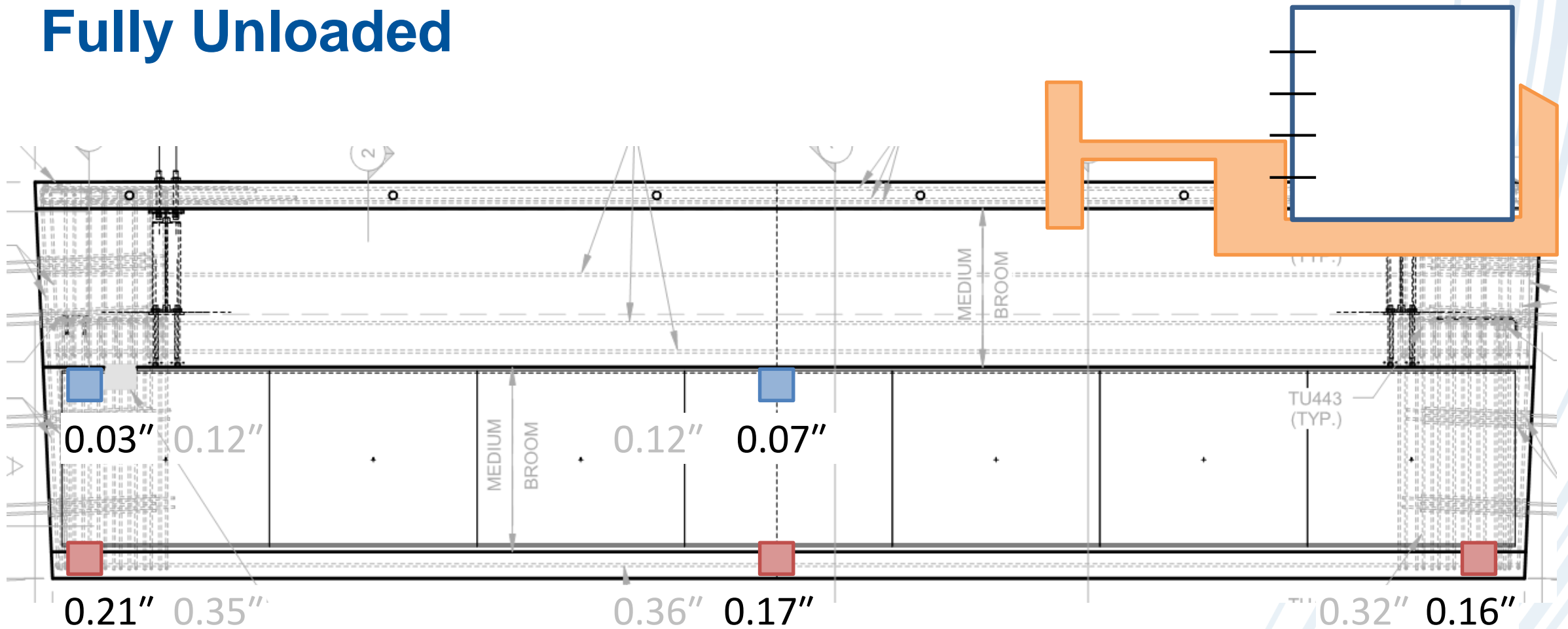
Last Loading Increment



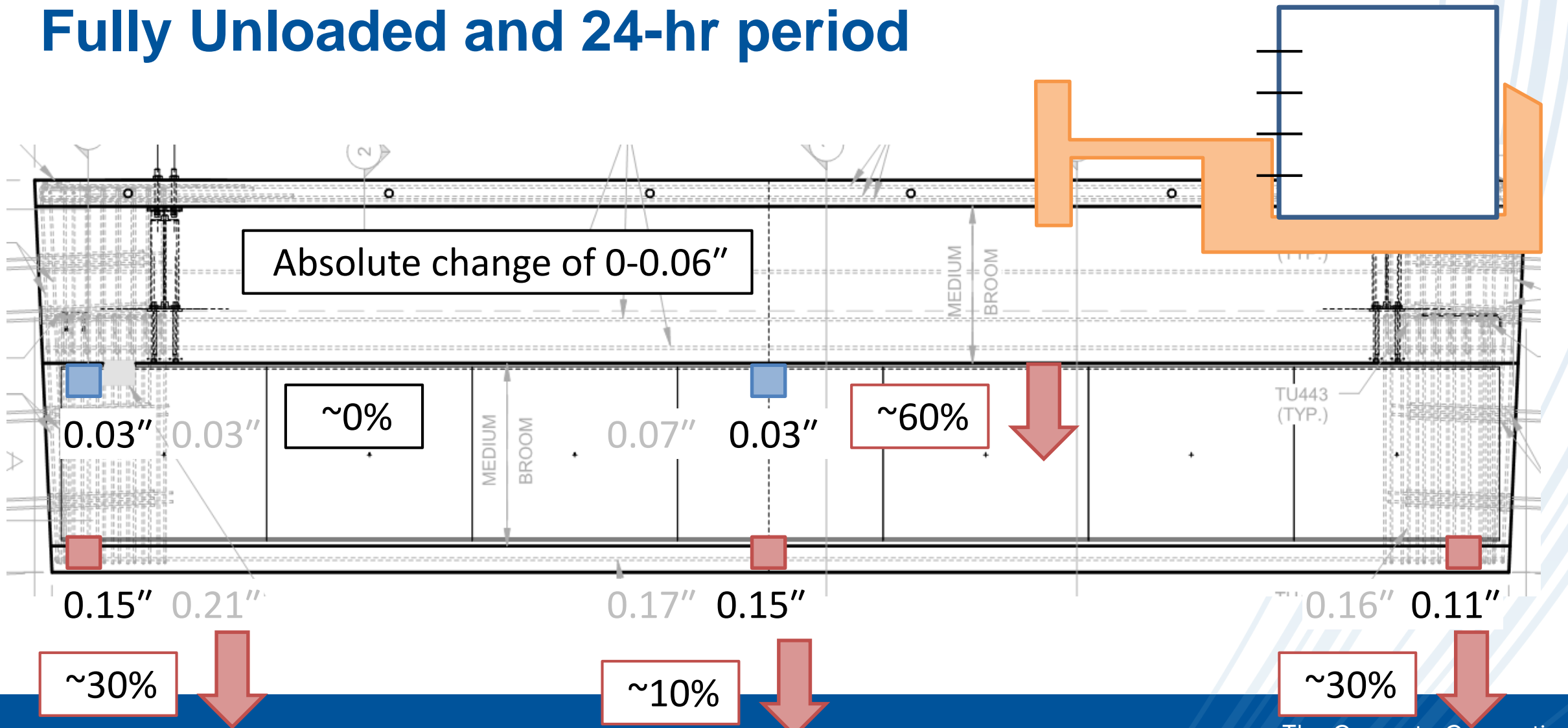
Last Loading Increment and 24-hr Hold



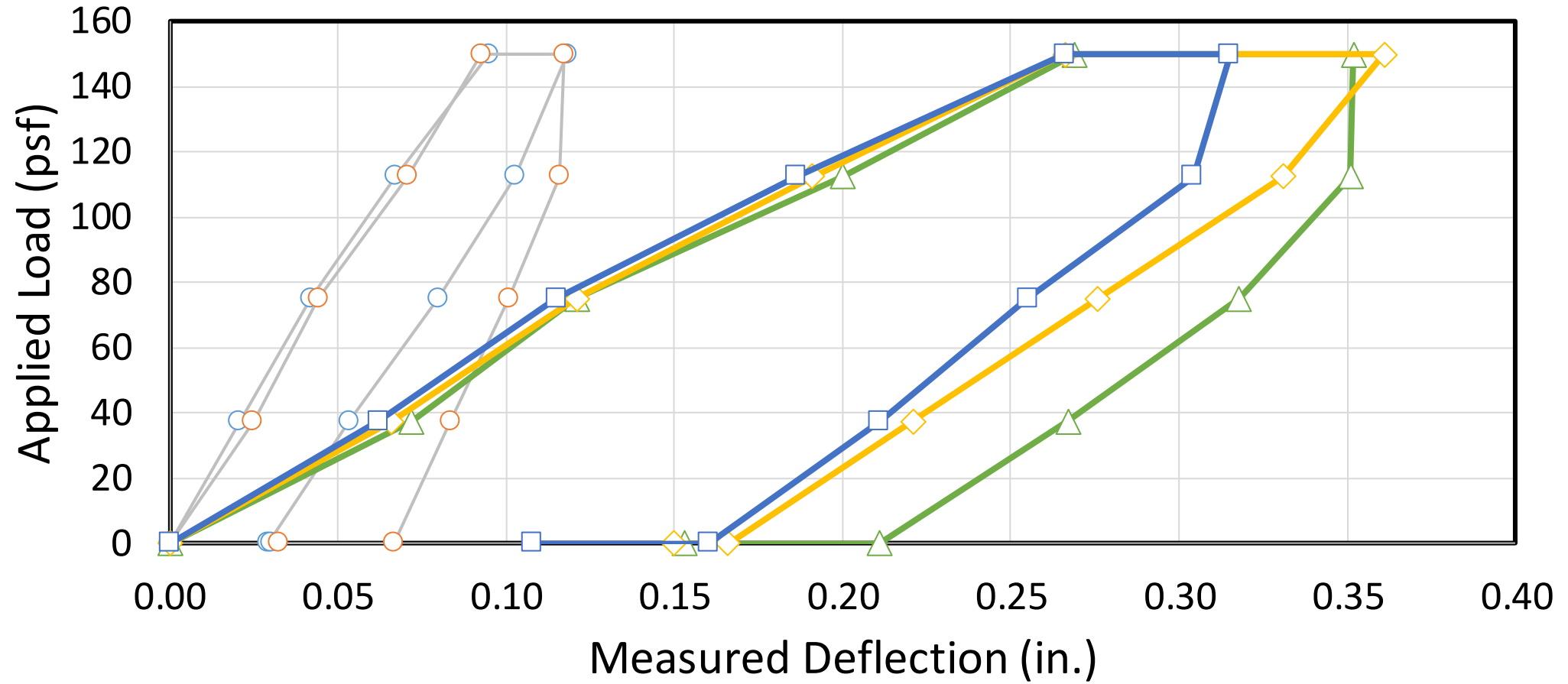
Fully Unloaded



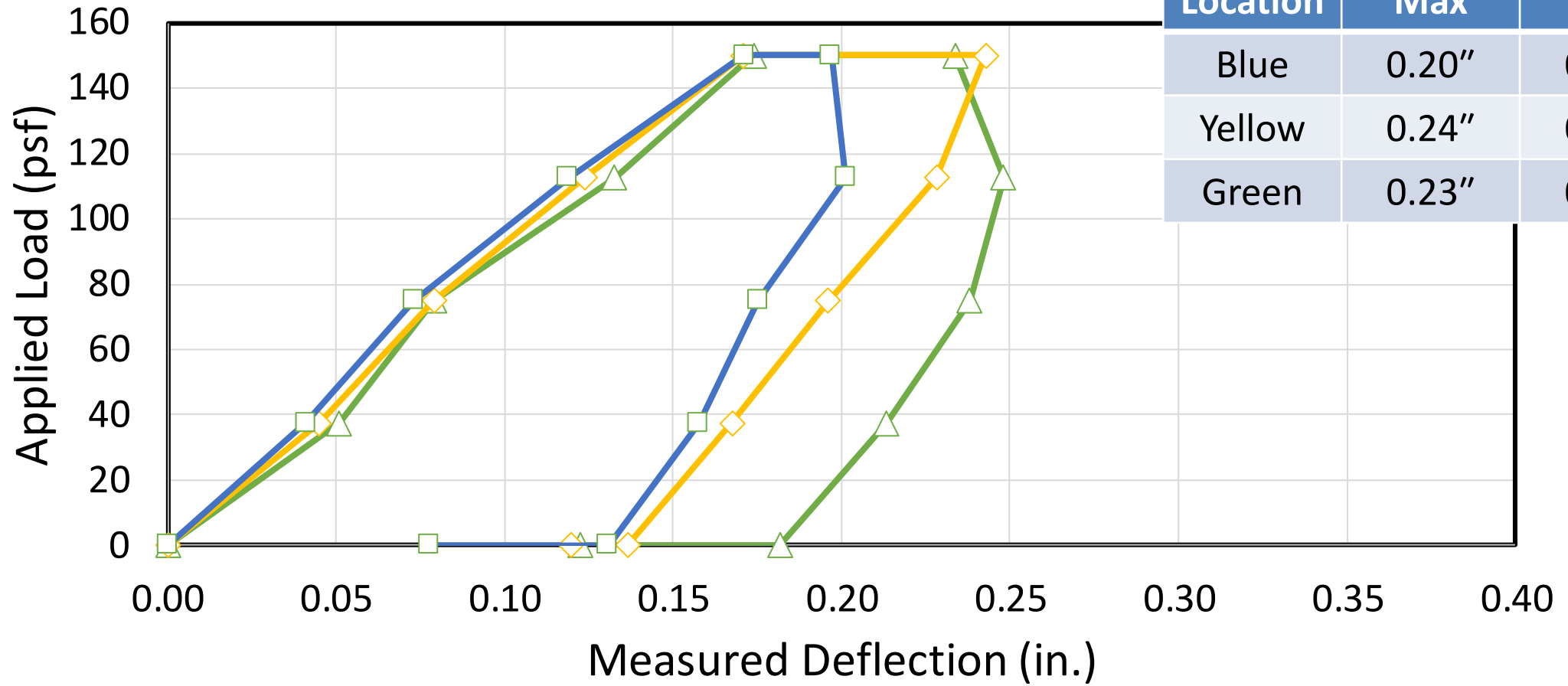
Fully Unloaded and 24-hr period



Deflections with Loading

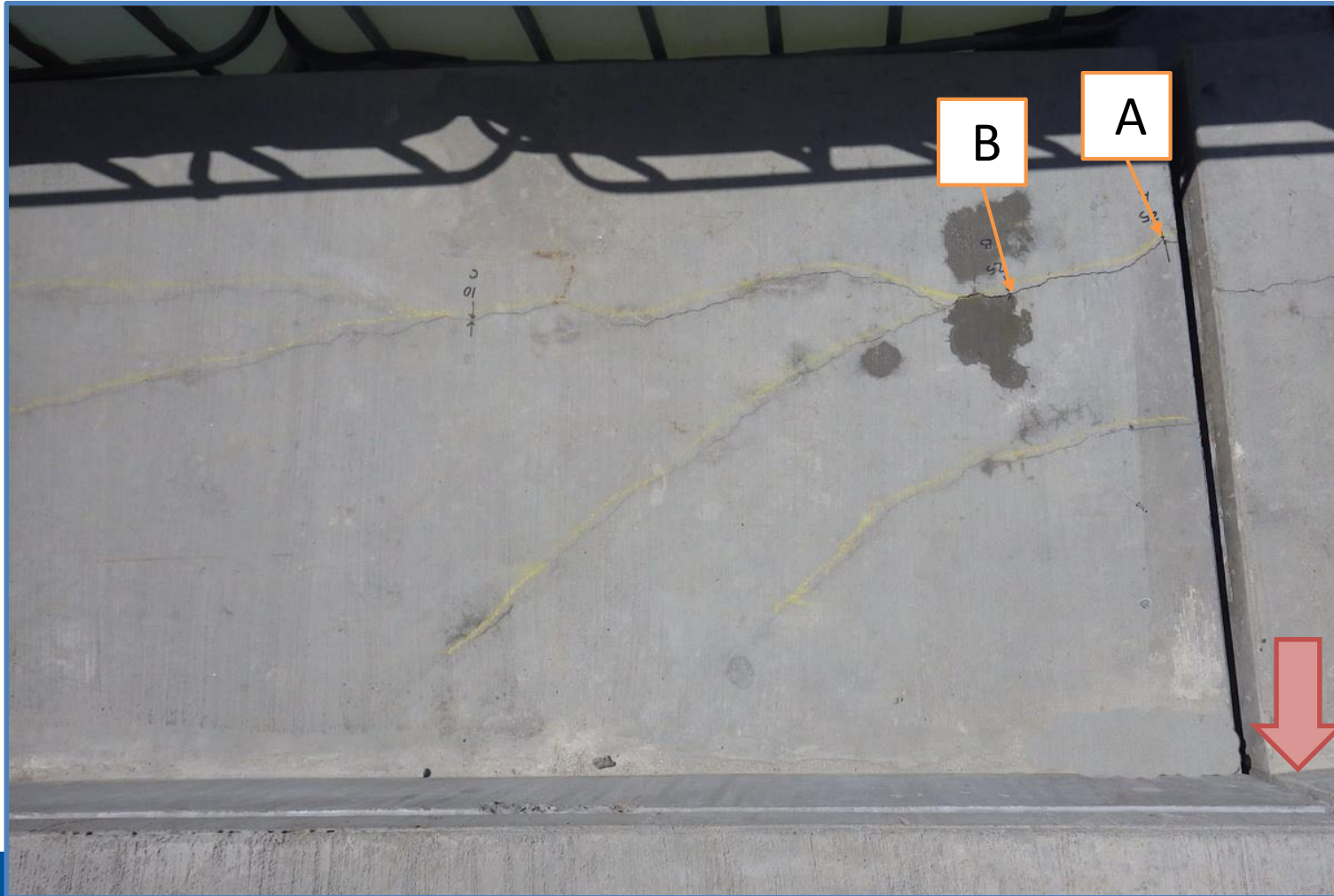


Relative Deflection



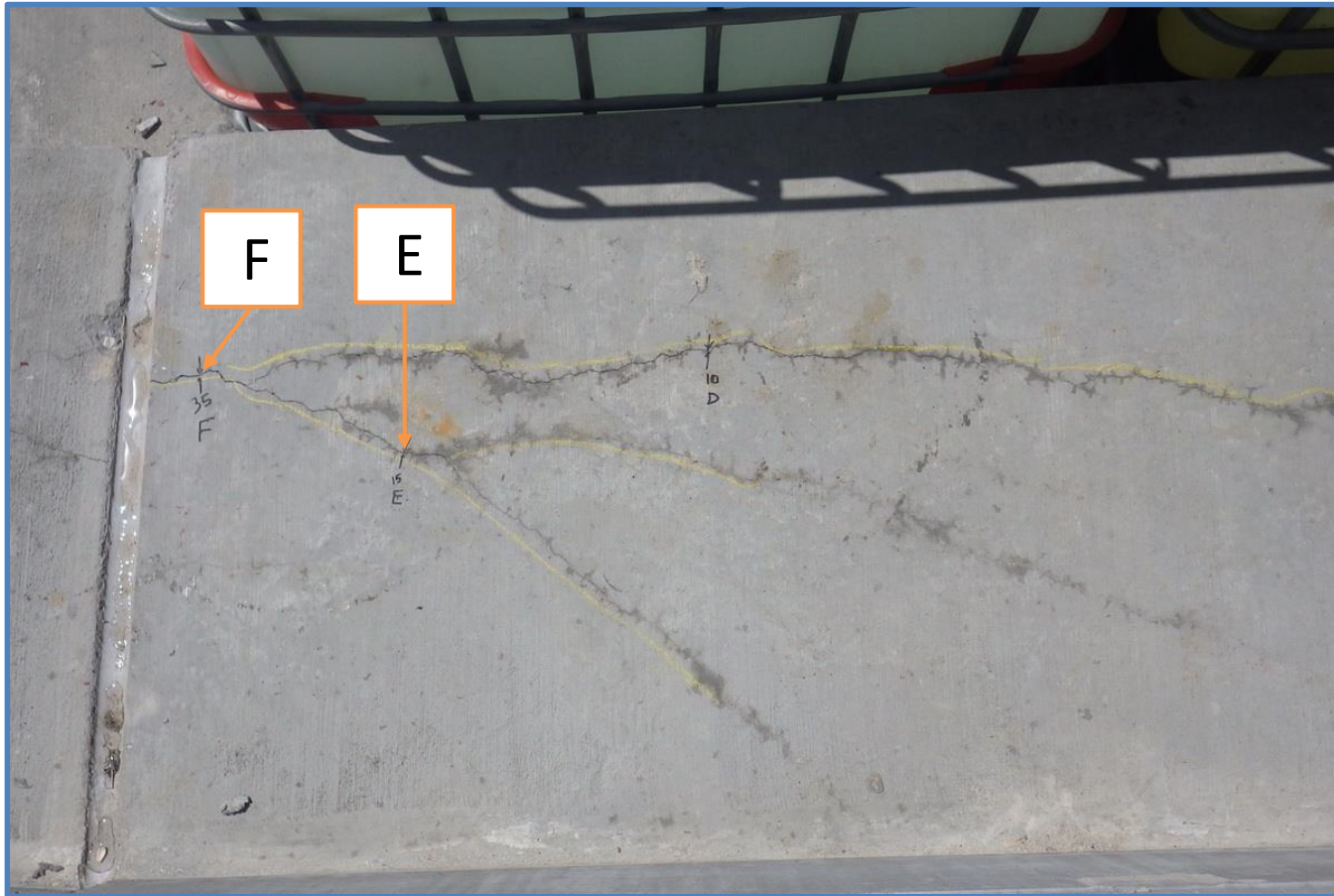
Location	Max	Final
Blue	0.20"	0.08"
Yellow	0.24"	0.12"
Green	0.23"	0.12"

Crack Width: West End



Description	A	B
Prior to test	25	25
Half Load	30	30
Full Load	35	35
Full Load + 24-hr hold	35	35
Half Load	35	35
No Load	30	30
No load + 24-hr period	30	30

Crack Width: East End



Description	E	F
Prior to test	15	35
Half Load	20	40
Full Load	20	45
Full Load + 24-hr hold	25	45
Half Load	25	45
No Load	25	40
No load + 24-hr period	20	40



Acceptance Criteria

- Maximum deflection (Eq. 20-1)

$$-\Delta \leq \frac{l_t^2}{20,000h} = \frac{(2 \times 48in)^2}{20,000(5.5in)} = 0.084in$$

- Residual deflection (Eq. 20-2)

$$-\Delta_r \leq \frac{\Delta_1}{4}$$

Criteria not satisfied

Location	Max
Blue	0.20"
Yellow	0.25"
Green	0.25"

Location	Max/4	Final
Blue	0.05"	0.05"
Yellow	0.06"	0.06"
Green	0.06"	0.06"

Discussion

- Dominated by cantilever bending
- Next steps
 - Retest?
 - Perform cyclic test?
 - Detailed analysis?
 - Retrofit?
- Other influences
 - Temperature on $\approx 20'$ cable



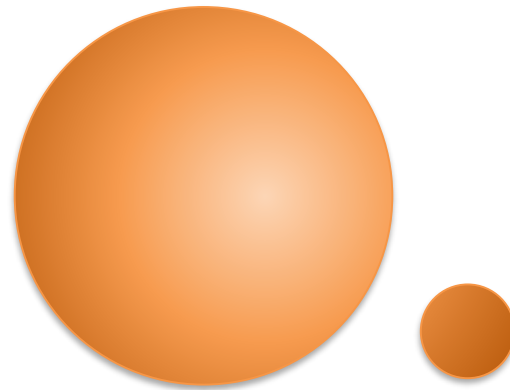
Corpus Christi Harbor Bridge Project



Main span: 1,661 ft (longest in US)

Confirm Development Length

- Develop yield strength of #20 (grade 75)
 - Diameter = 2.5"
 - Min. yield strength = 368 kips
- Supplied by Williams Form Engineering
 - All-thread bars
 - Threaded terminator
- Footings

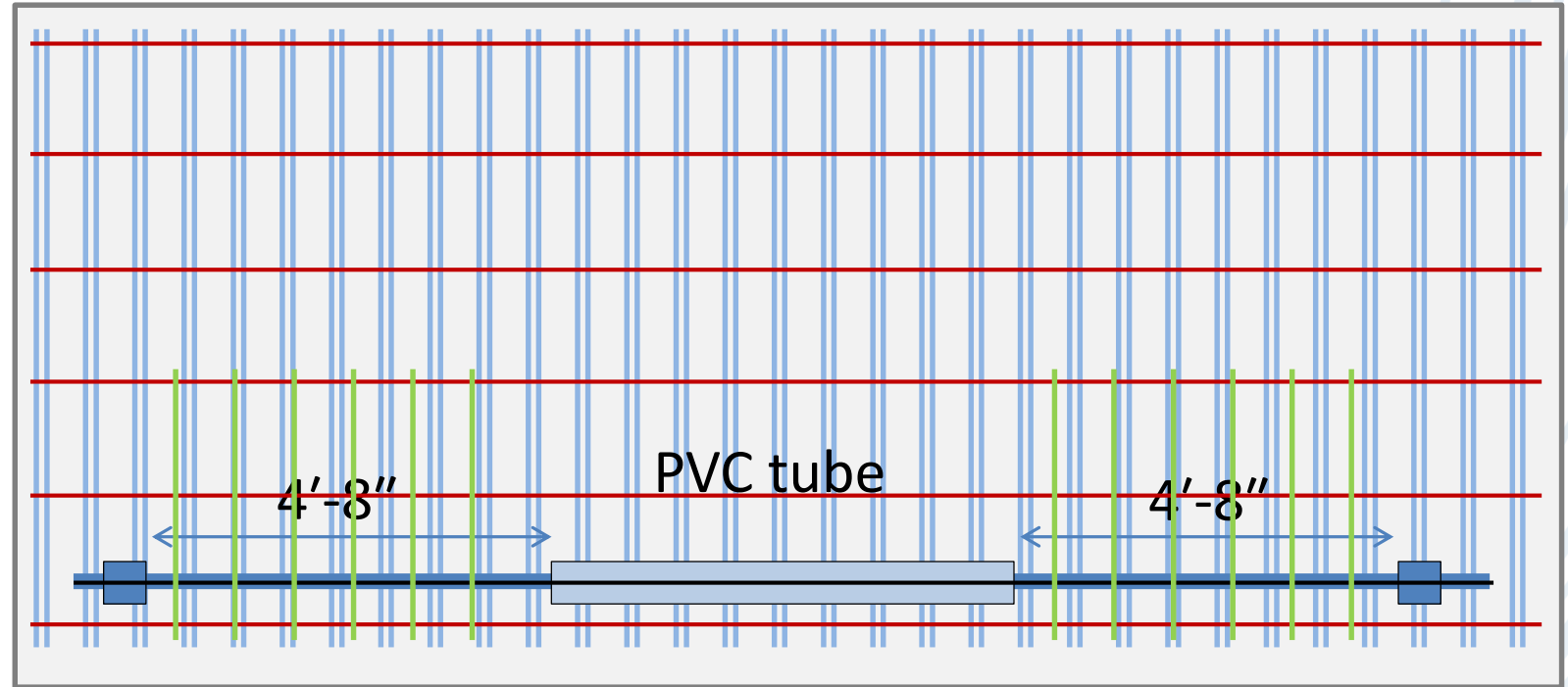
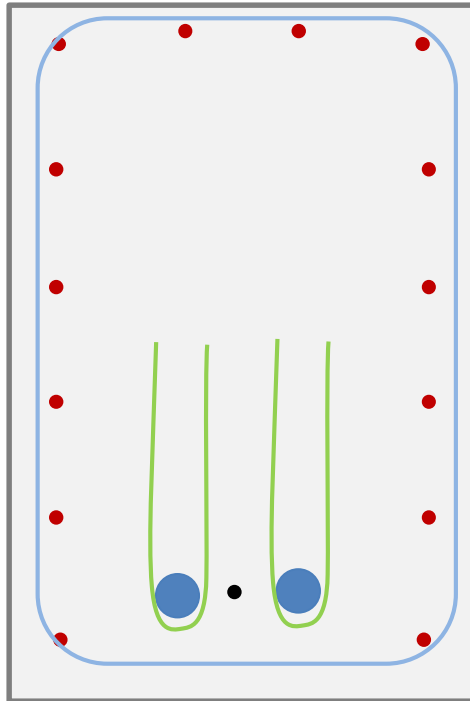


#20 ~ 16#5s



Fabricated Beams

- 6'-0" deep, 4'-0" wide, 15'-6.5" long
- Development length: 4'-8"



2#20s

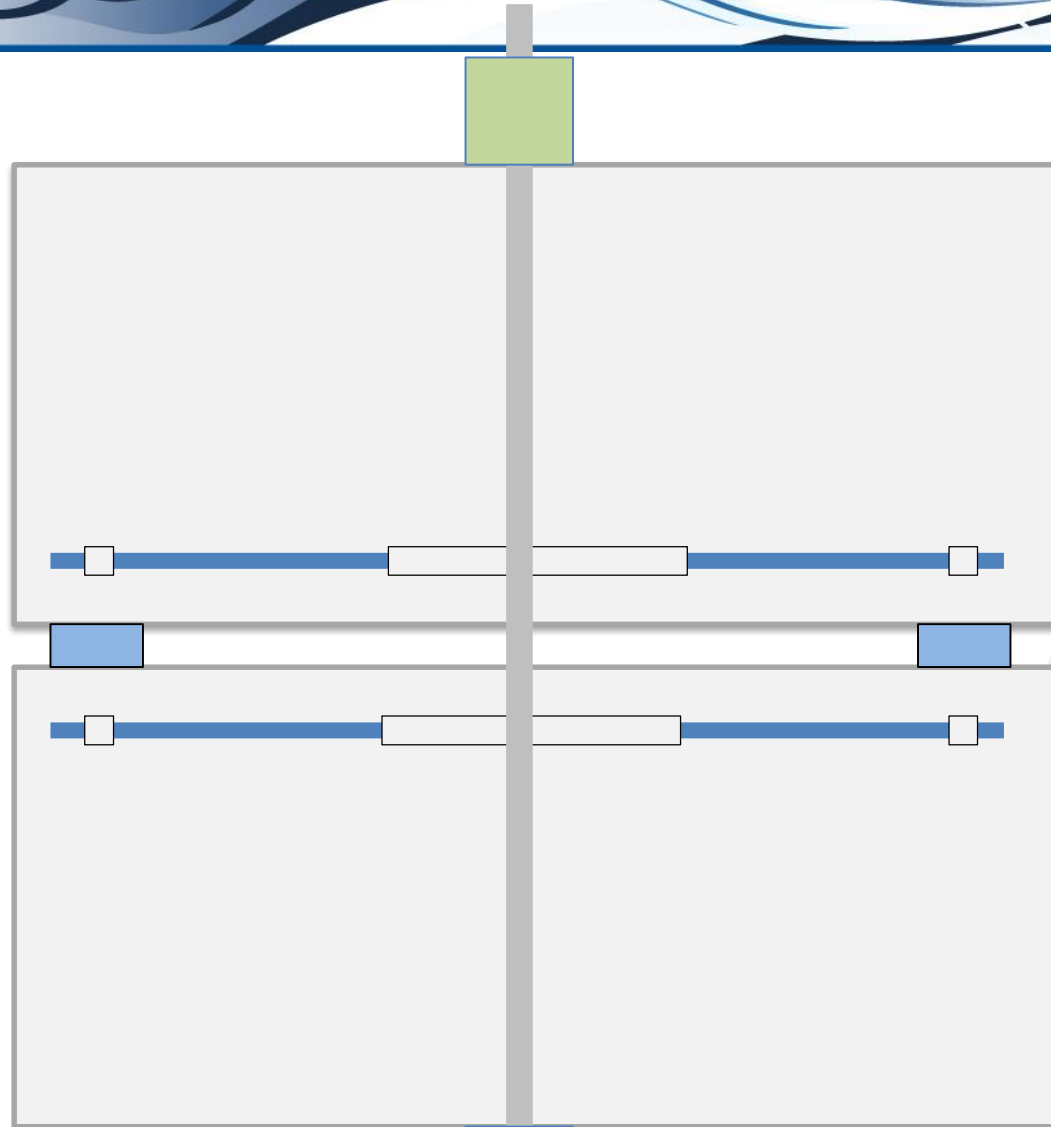
1#5 (Grade 100)

14#5

2#6 @ 6" O.C.

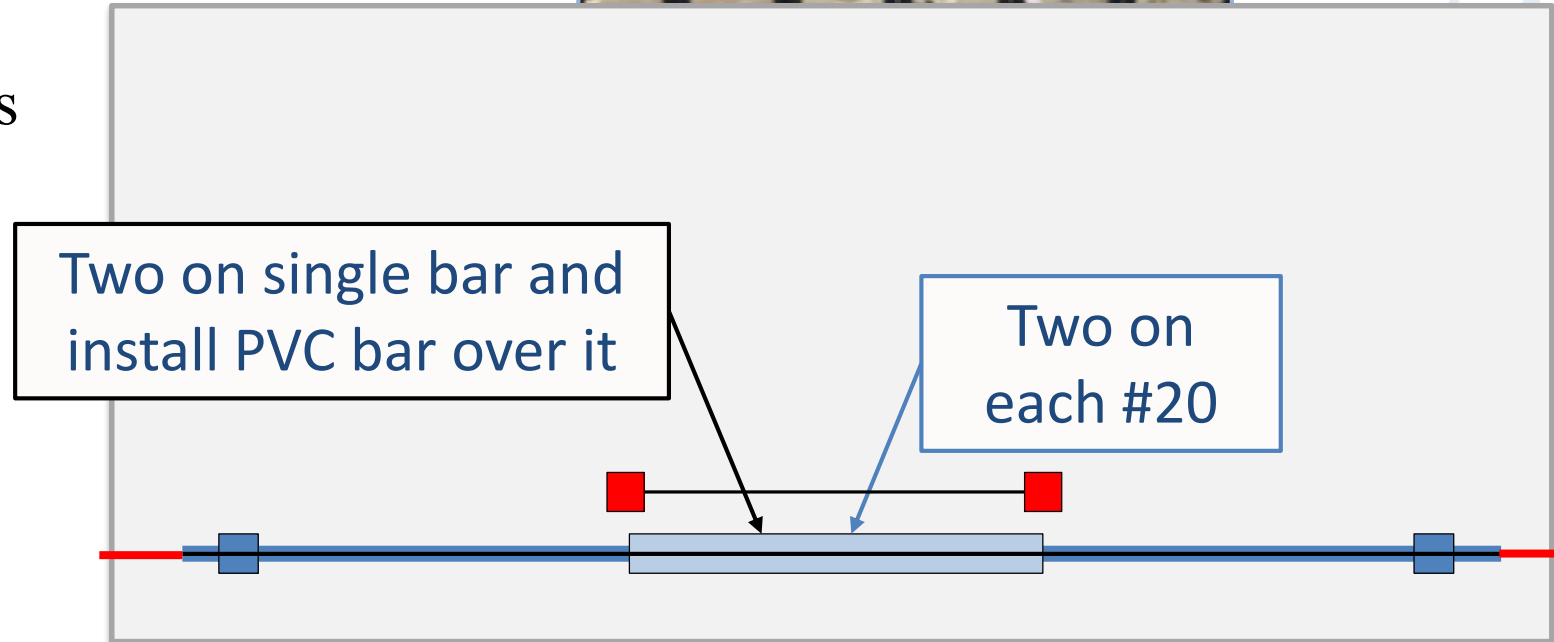
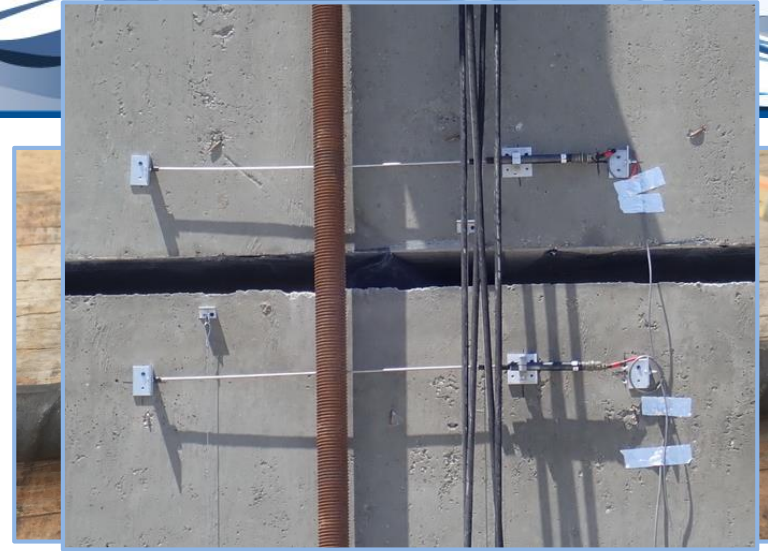
#4 @ 9" O.C.

Two Tests at Once



Instrumentation Plan

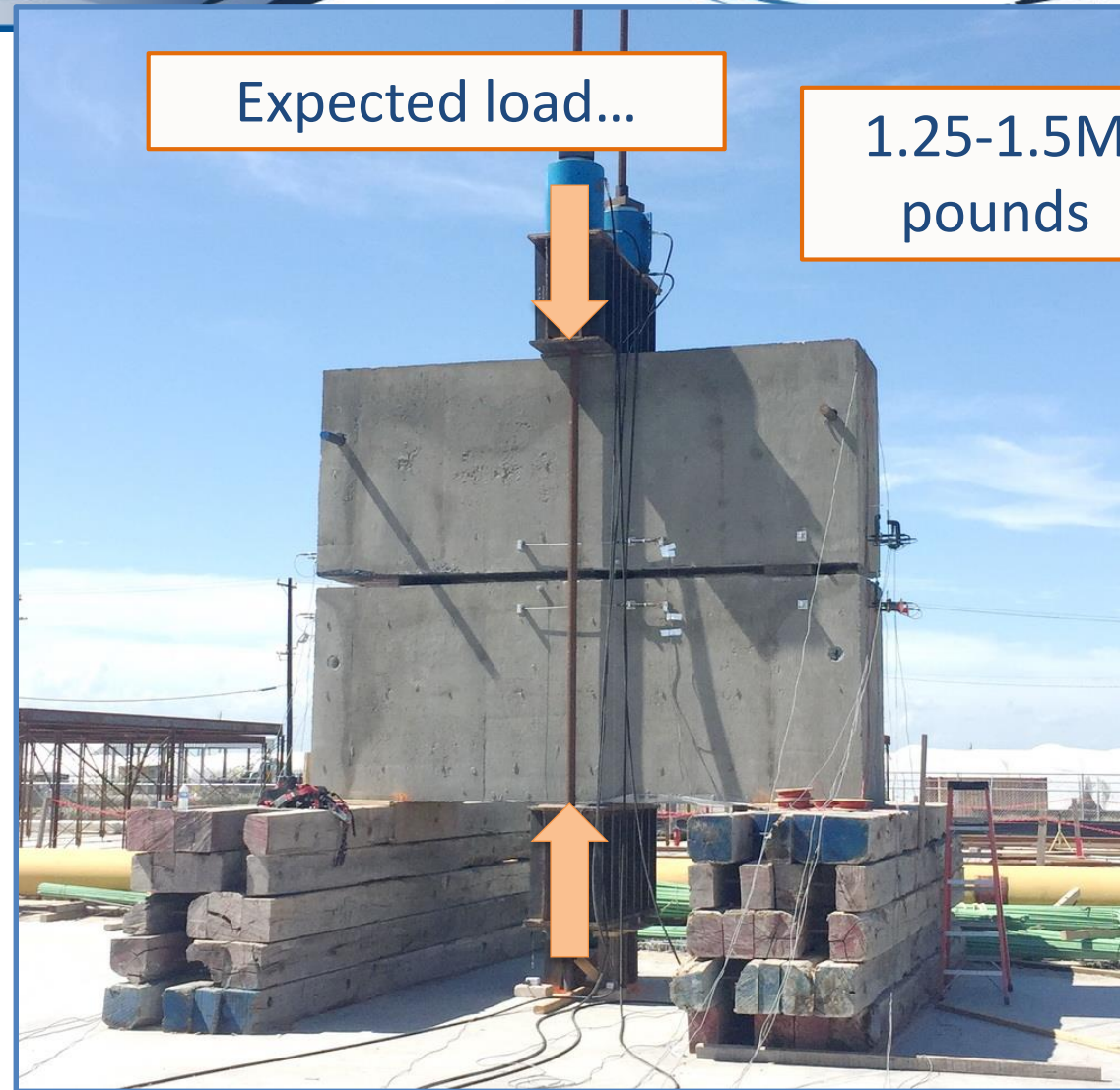
- Internal strain gages
 - Vibrating wire gages (#5 bar)
 - Foil strain gages (#20 bar)
- External strain gages
- Slip at ends of #20 bars
- Deflection
- Pressure



Acceptance Criteria

- Reach 75 ksi in #20 bars
 - (2800 microstrain)
- Slip < 0.04"
- No evidence of bond failure

- 50' safety buffer



Strain in #20 Bars

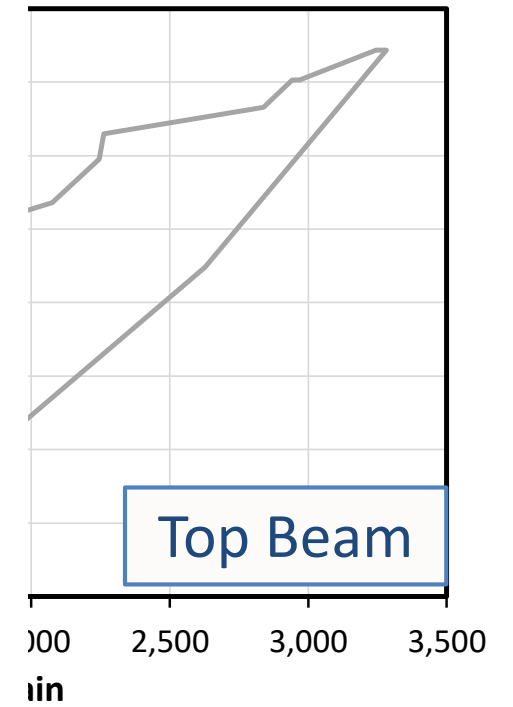
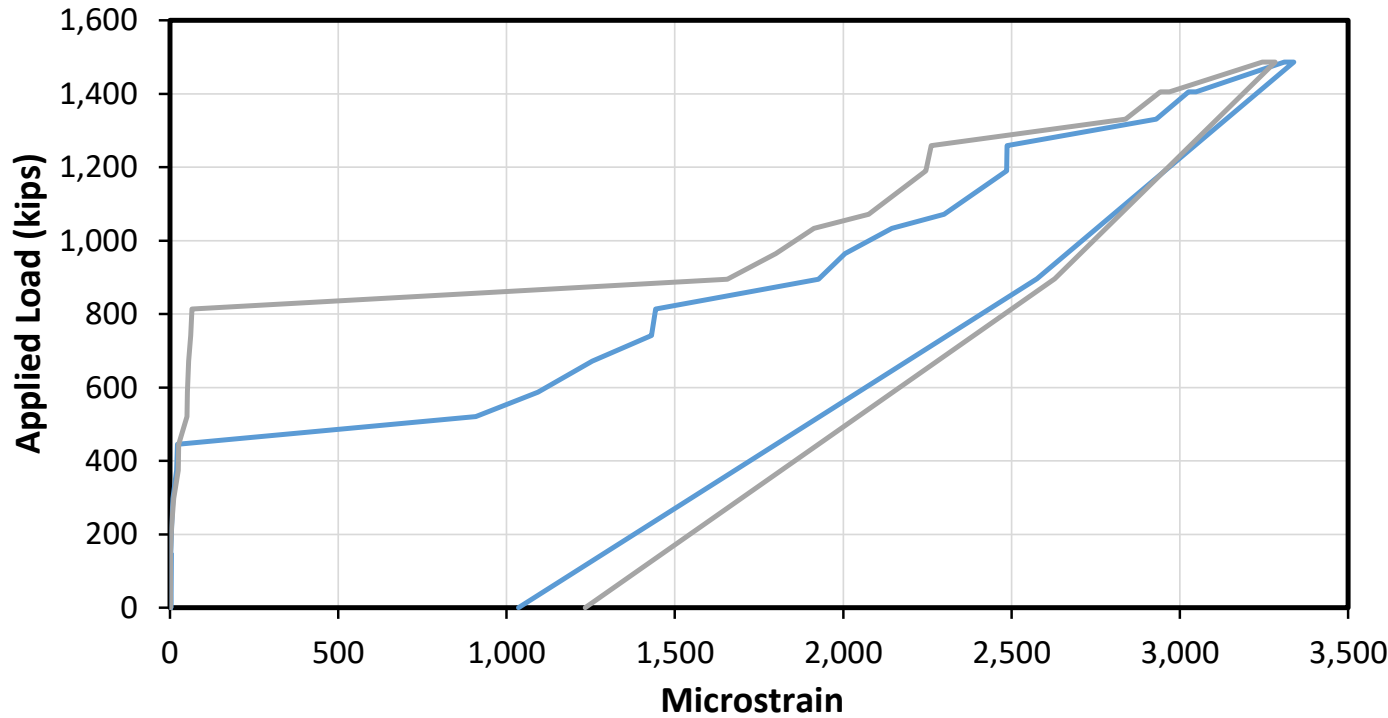
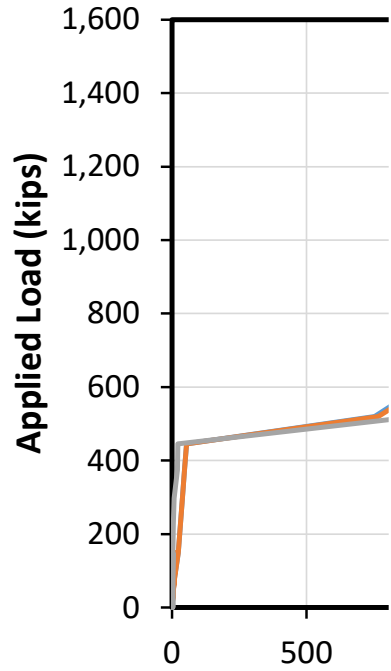
Start loading

Loud bang

Continue loading,
another bang

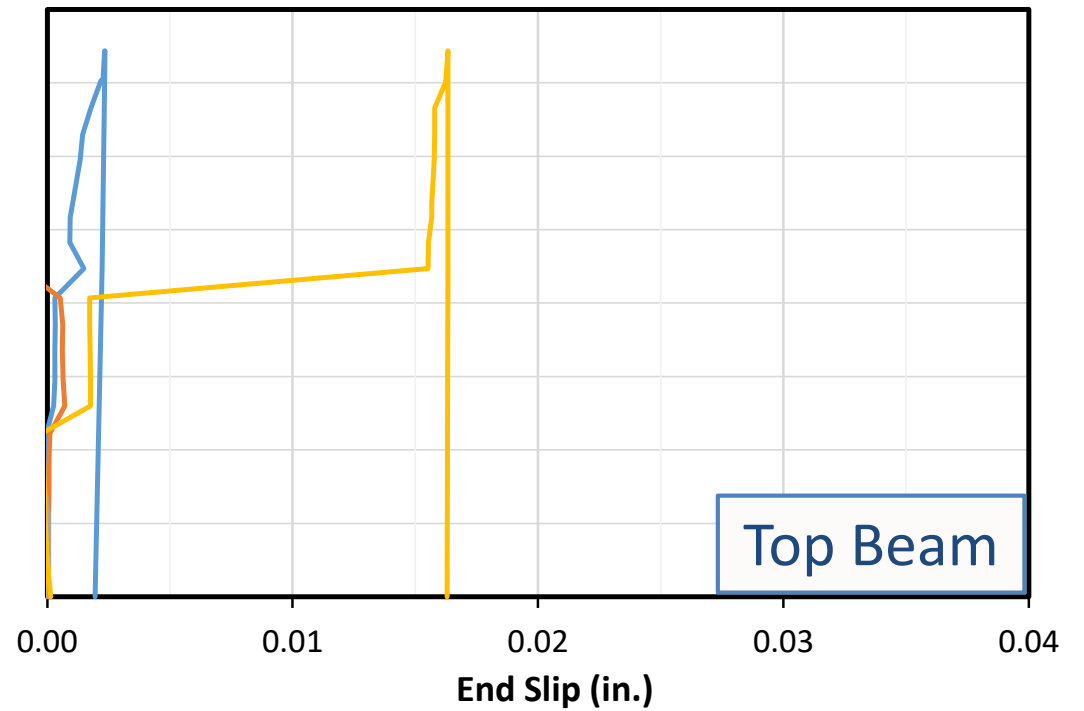
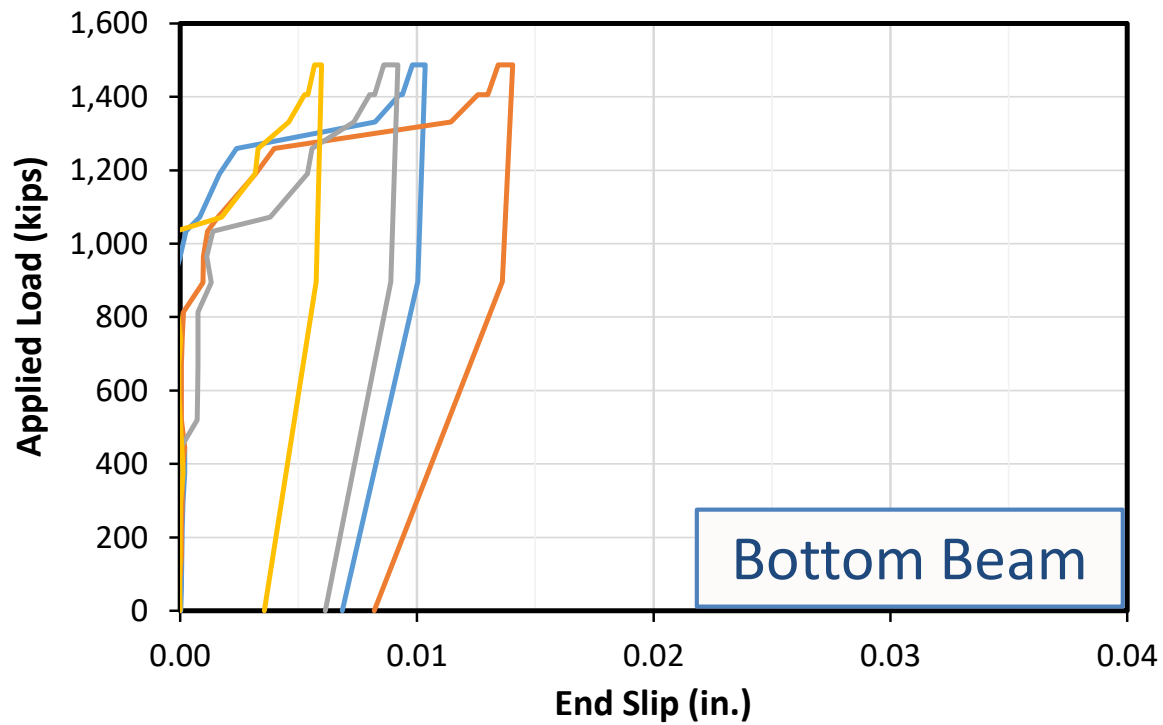
Continue loading

Unload



Top Beam

Slip at Ends



Condition of Beam



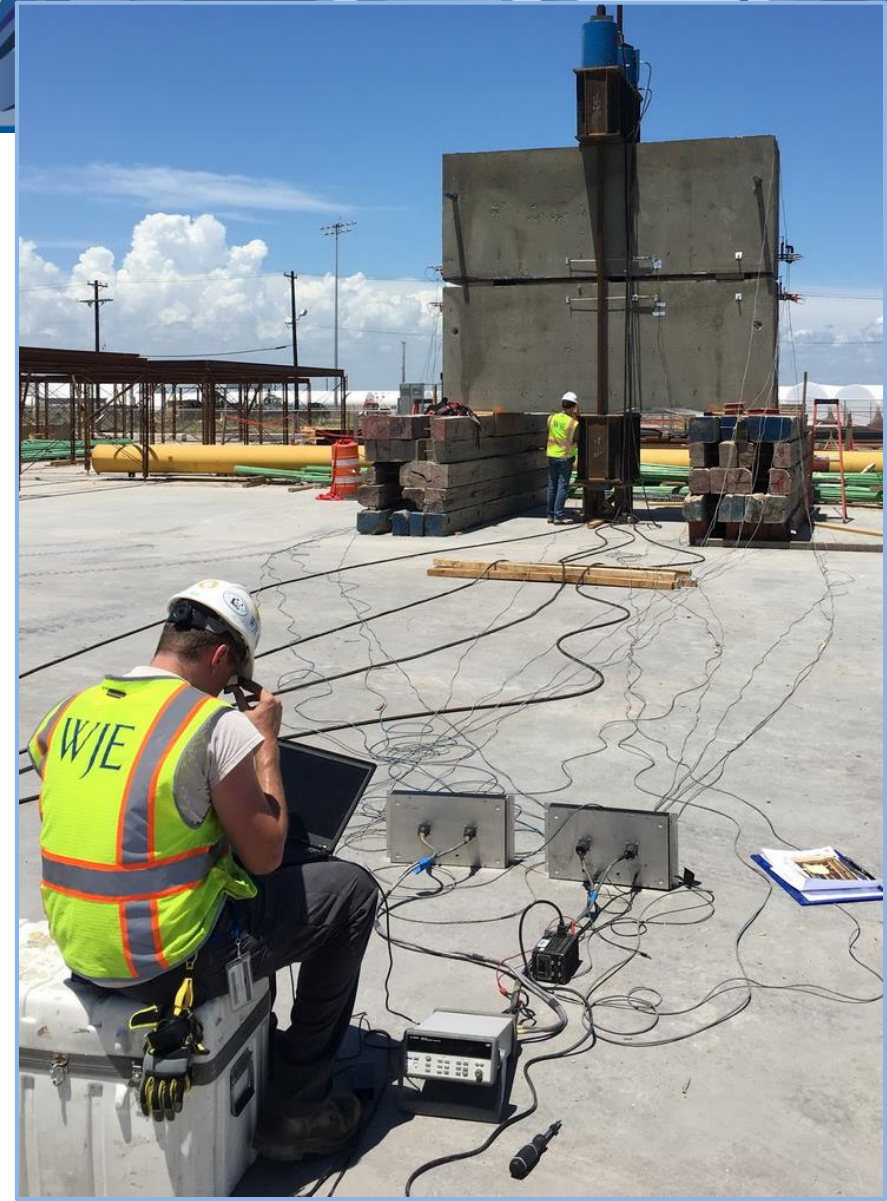
Residual Strain (external): $1,000 \mu\epsilon$
 $0.04'' \text{ crack} / 38'' \text{ gage} = 1,052 \mu\epsilon$



Residual Strain (external): $1,200 \mu\epsilon$
 $0.05'' \text{ crack} / 38.125'' \text{ gage} = 1,300 \mu\epsilon$

Discussion

- Met acceptance criteria
- Idealized behavior
- Important to measure right component
- Redundancy
 - Gages
 - Equipment



Questions?

