

# Thermal Strains in Concrete Pavements – Measurements and Field Implementation

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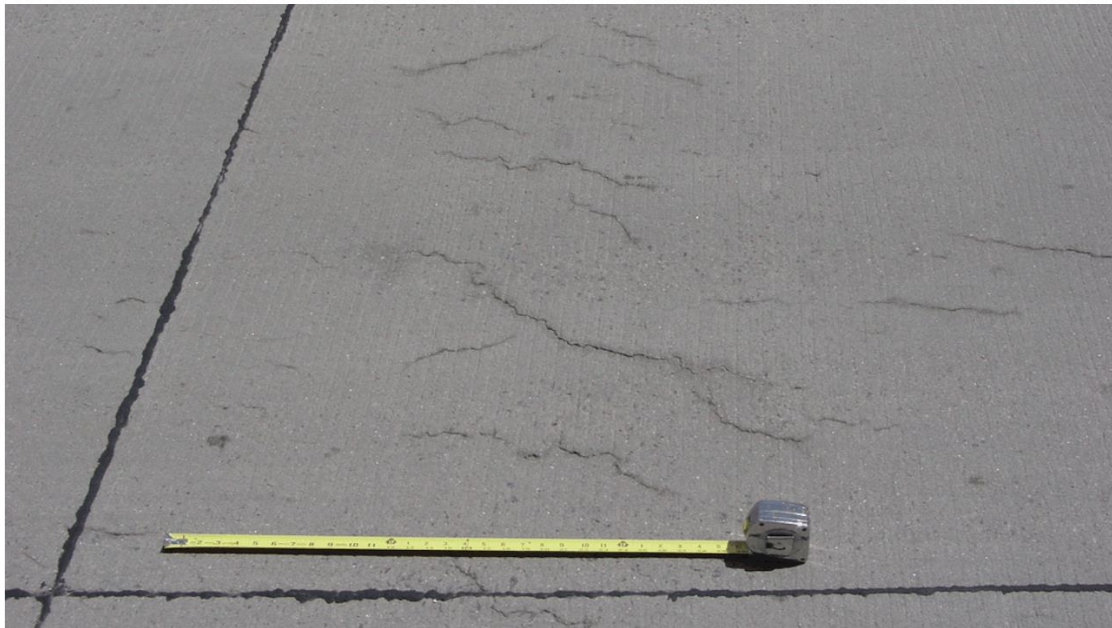
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<http://enkoat.com>





# Introduction

**Ordinary Portland Cement**



**Water**



**Aggregate**



**Dams**



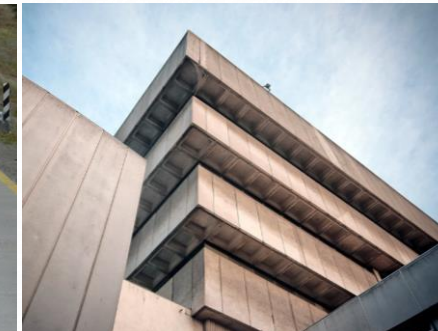
**Bridges**



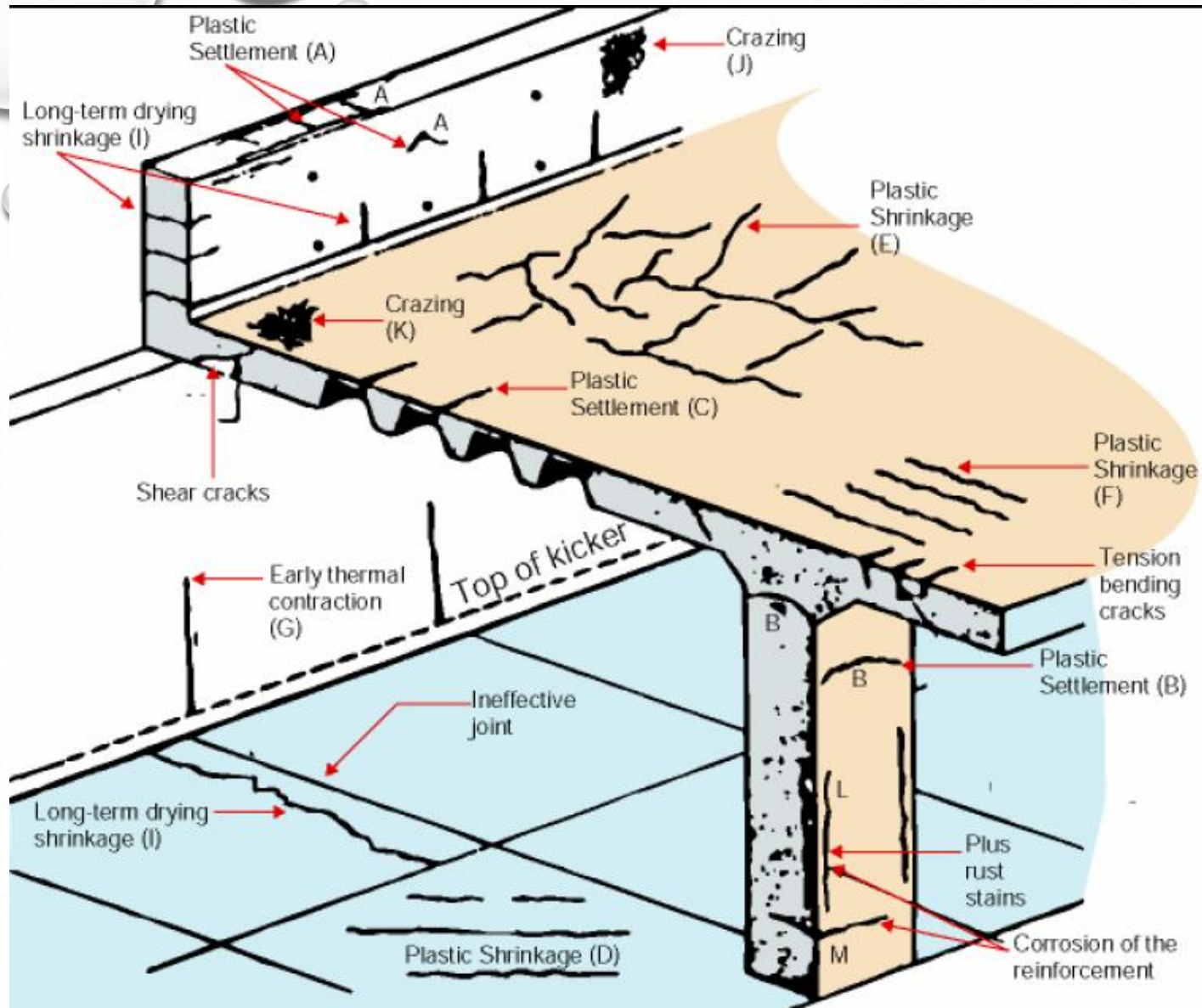
**Roadways**



**Buildings**



# Types of Cracking



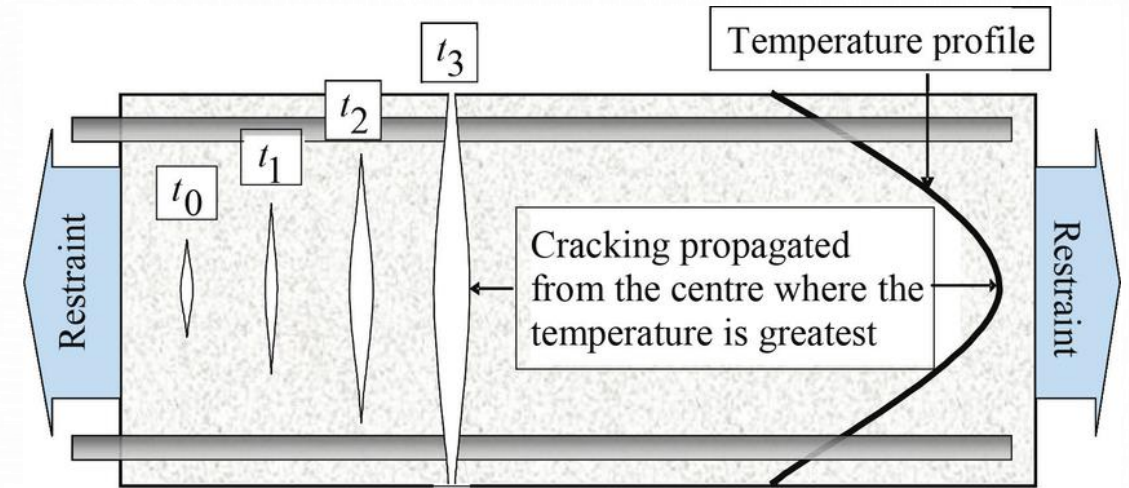


# Thermal cracking

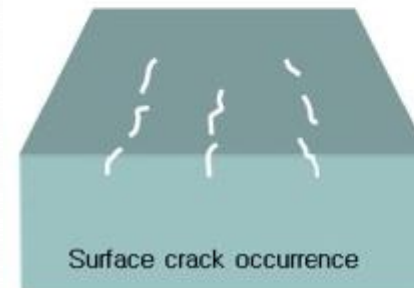
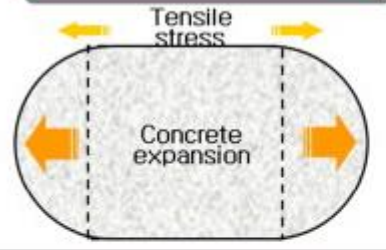
- Hydration of cement is an exothermic process it generates heat
- As the concrete cools it contracts and in extreme conditions may contract in three days as much due to cooling as it could in a year due to drying conditions
- A temperature differential of  $35^{\circ}$  F within 1 ft is usually considered enough to cause cracking
- However, within 24-hours of placement, concrete temperatures can reach anywhere from  $20^{\circ}$  to  $50^{\circ}$  F hotter than ambient temperatures

# Motivation

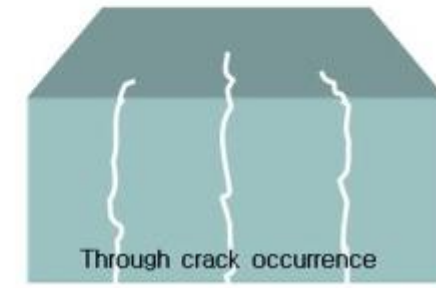
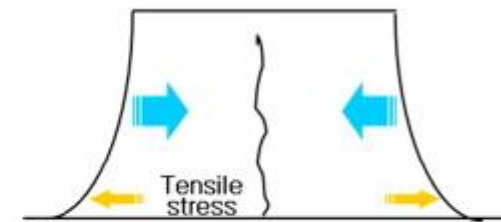
## Thermal Related Damage



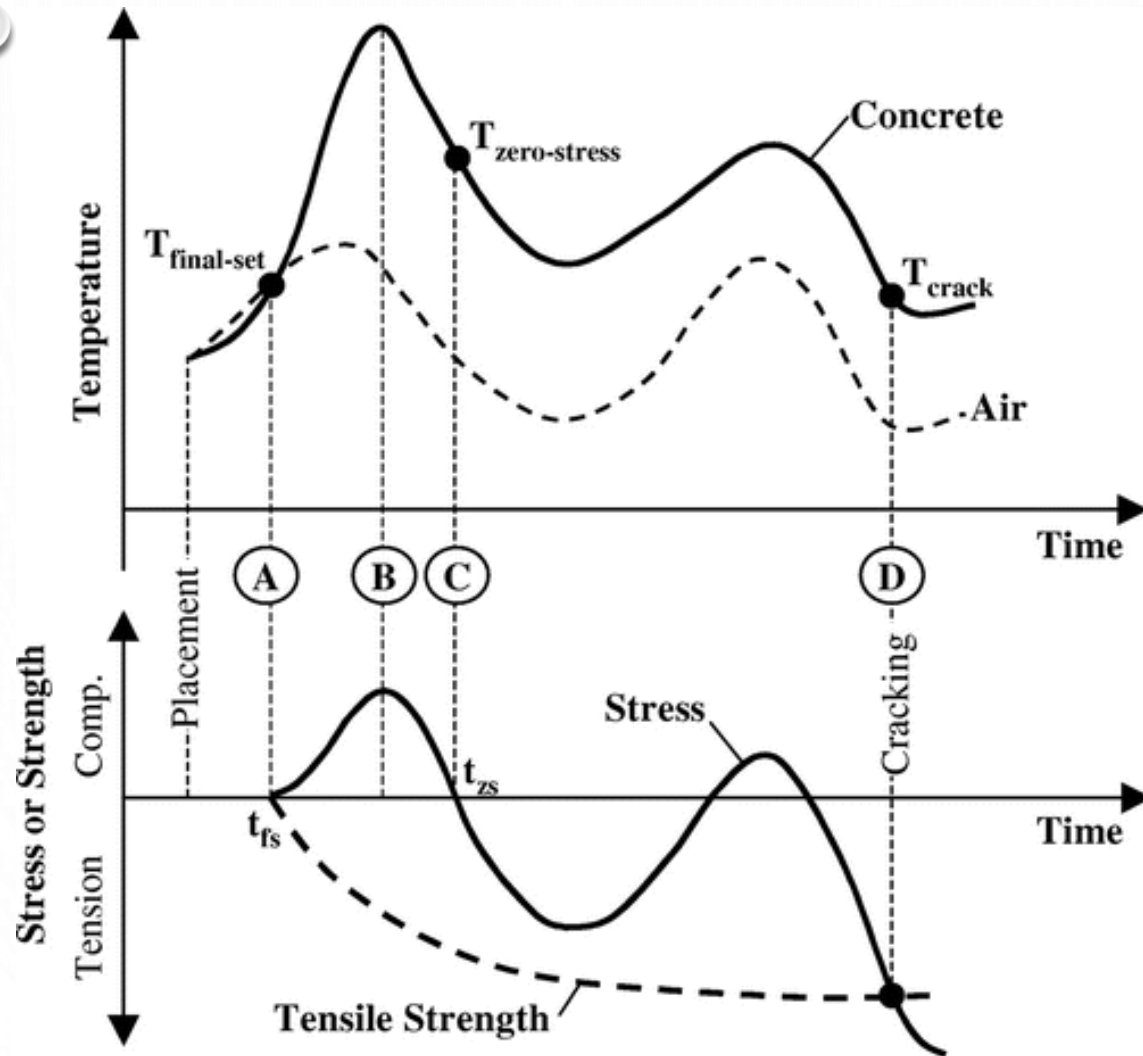
### Internal restraint



### External restraint



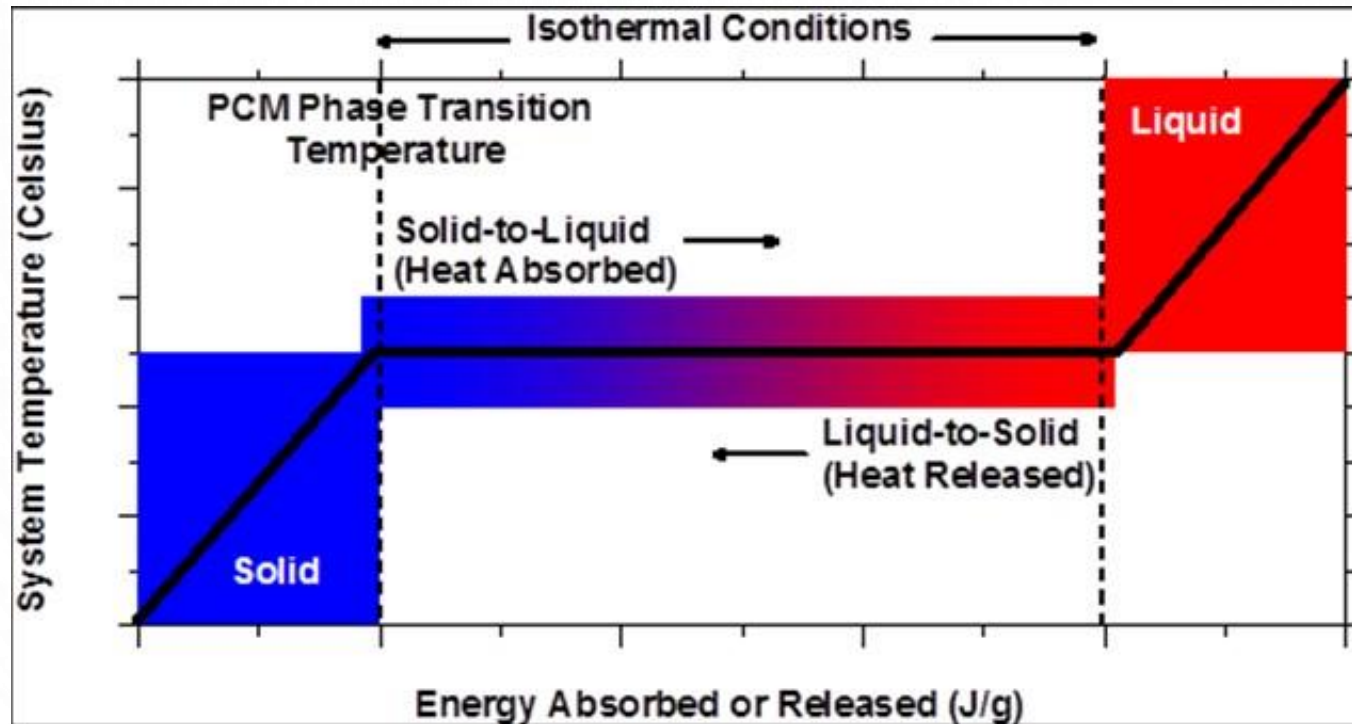
# Early age cracking in infrastructure





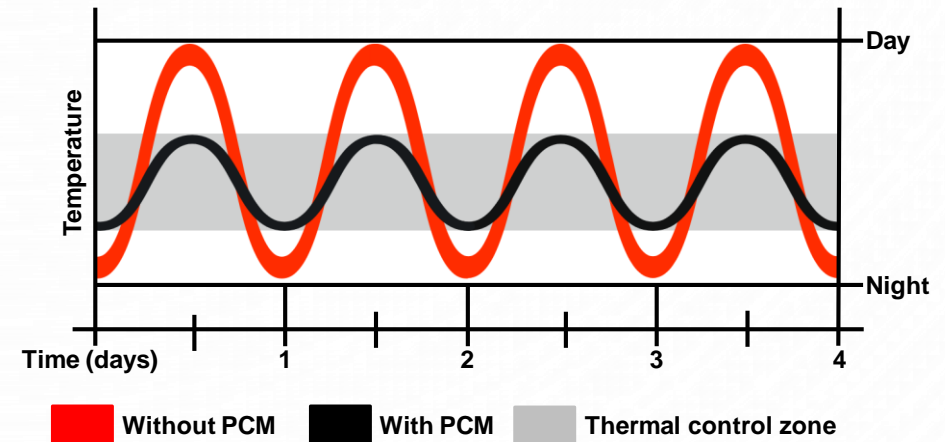
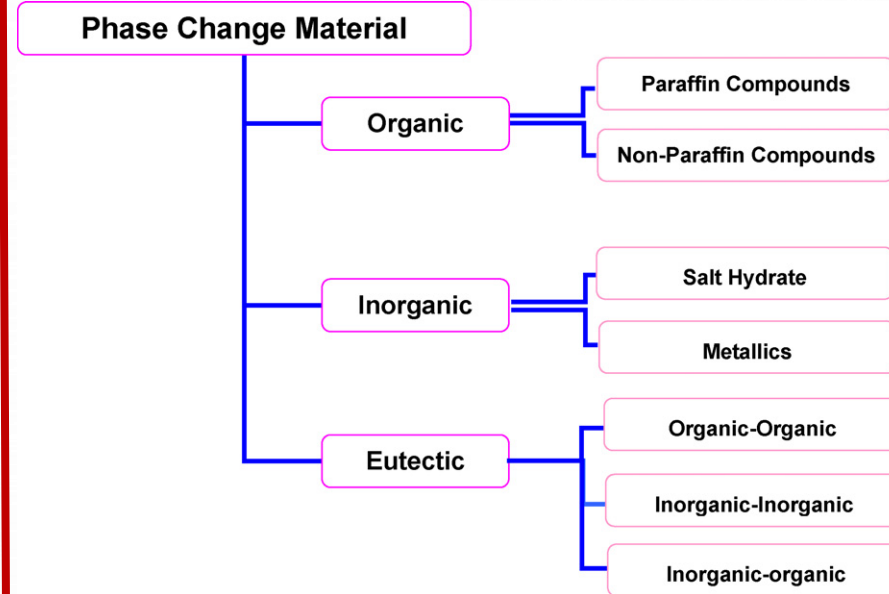
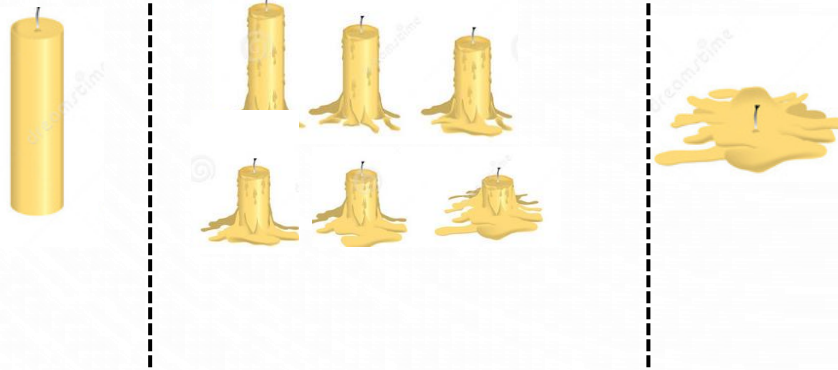
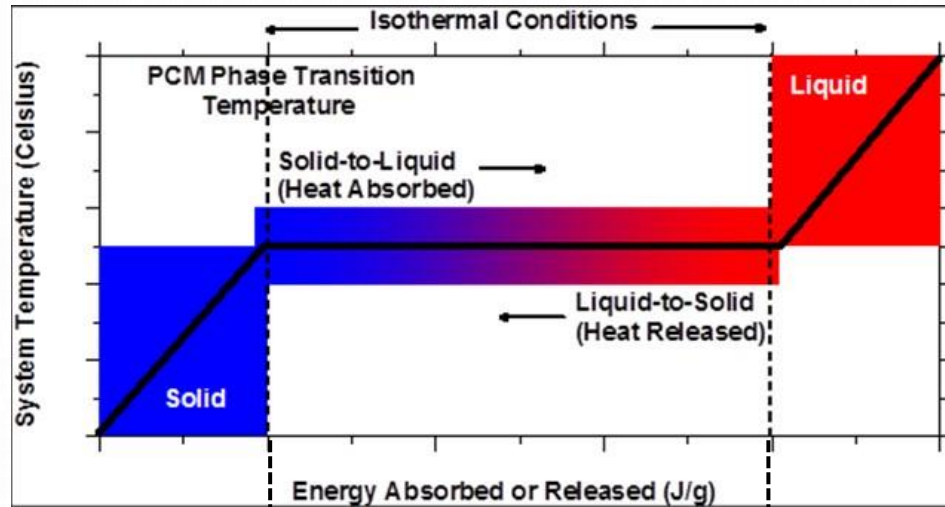
# One Active Mitigation Strategy - PCMs

- Phase change materials (PCM) are combined sensible-and-latent heat materials that can be used to store and dissipate energy in the form of heat

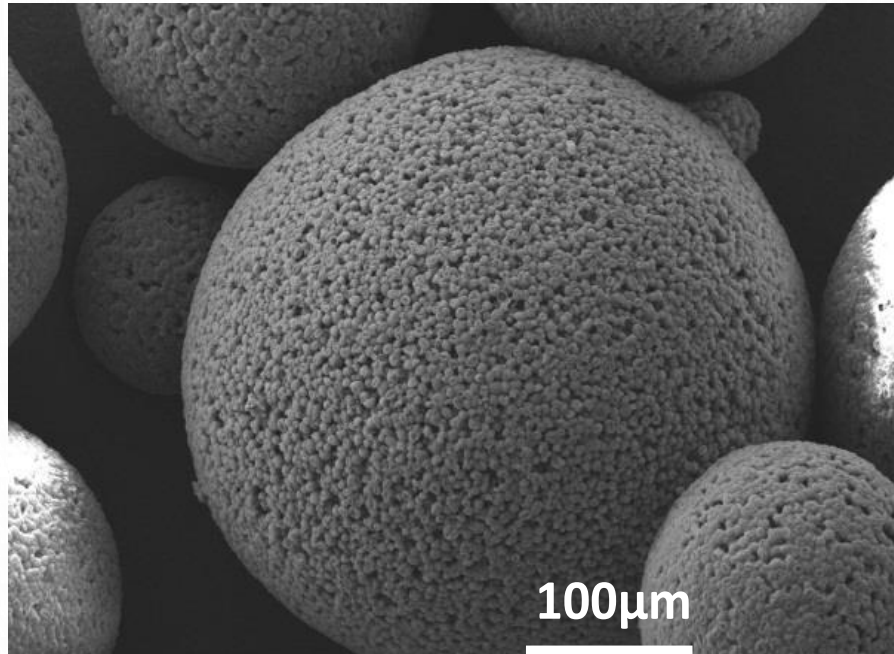




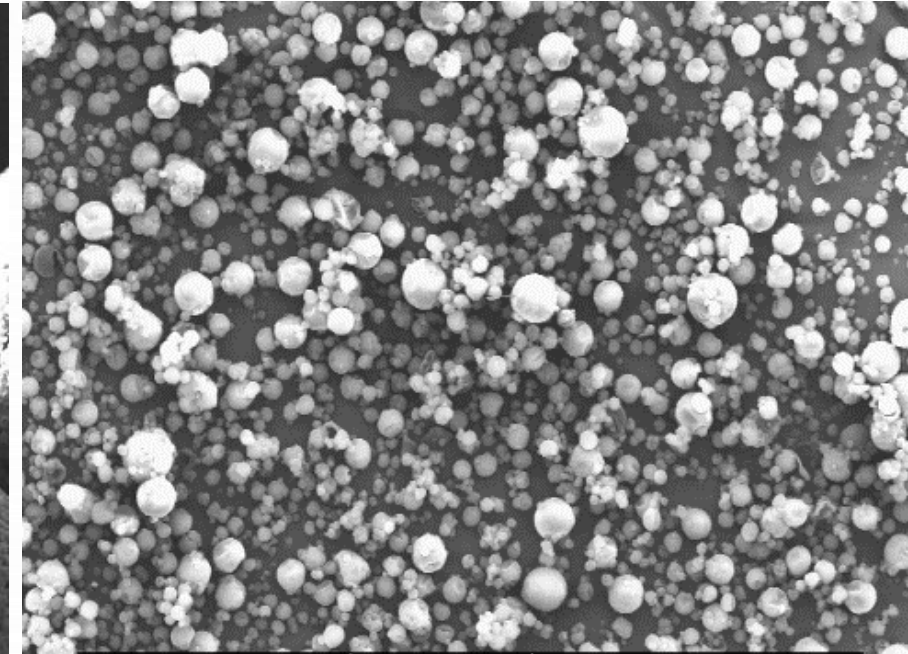
# Background



# Microencapsulated PCM Type



**PCM-M**



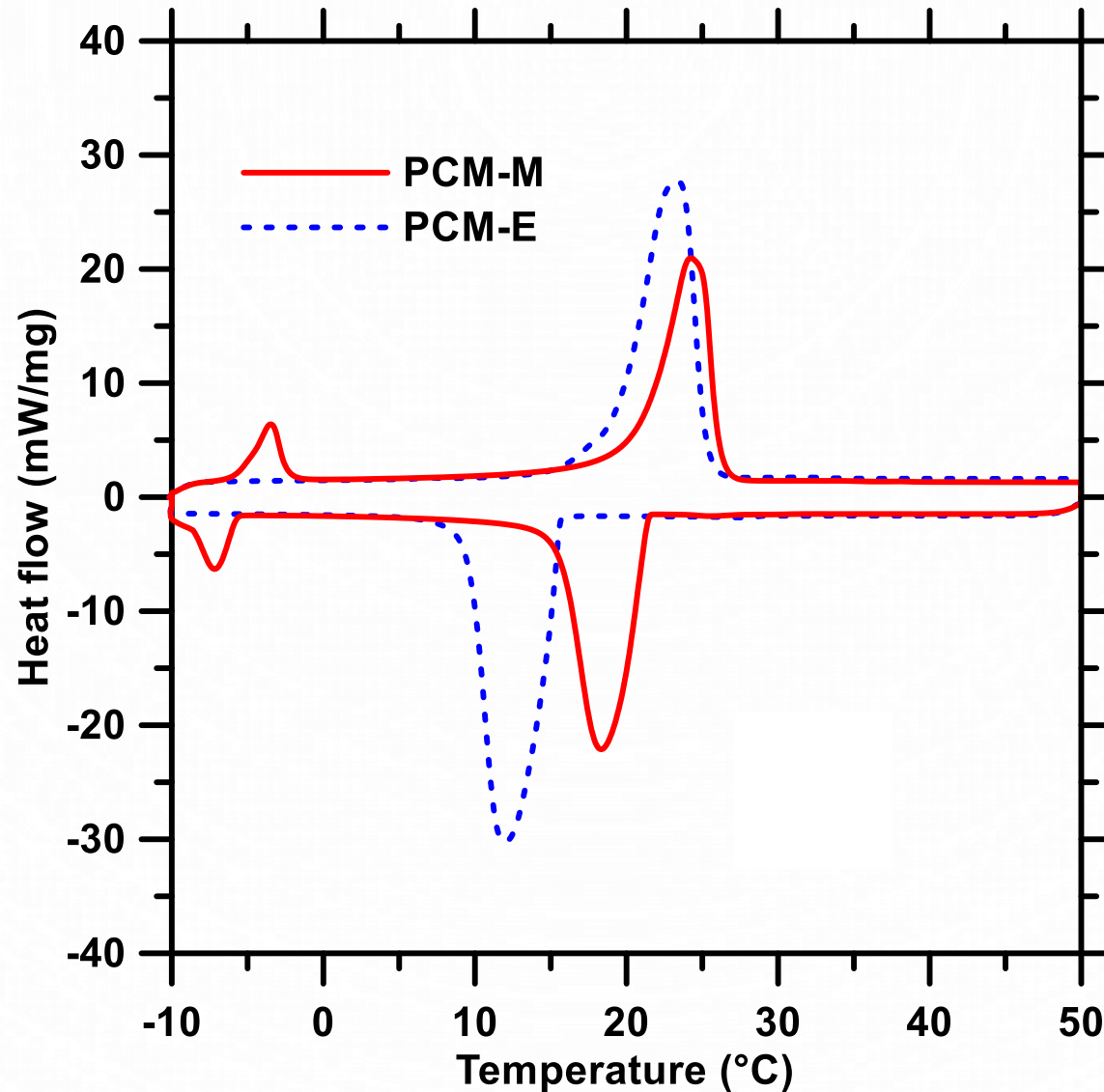
**PCM-E**

**100µm**

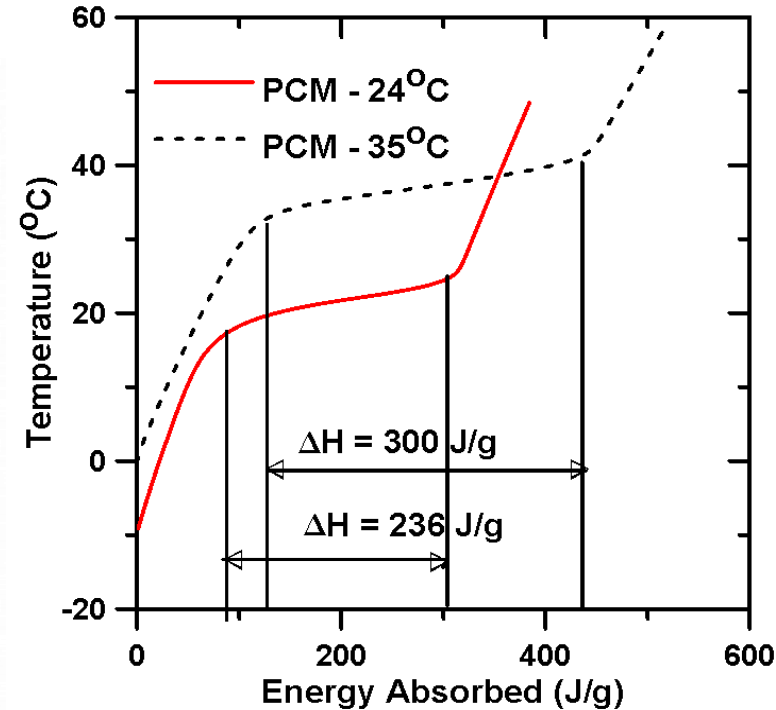
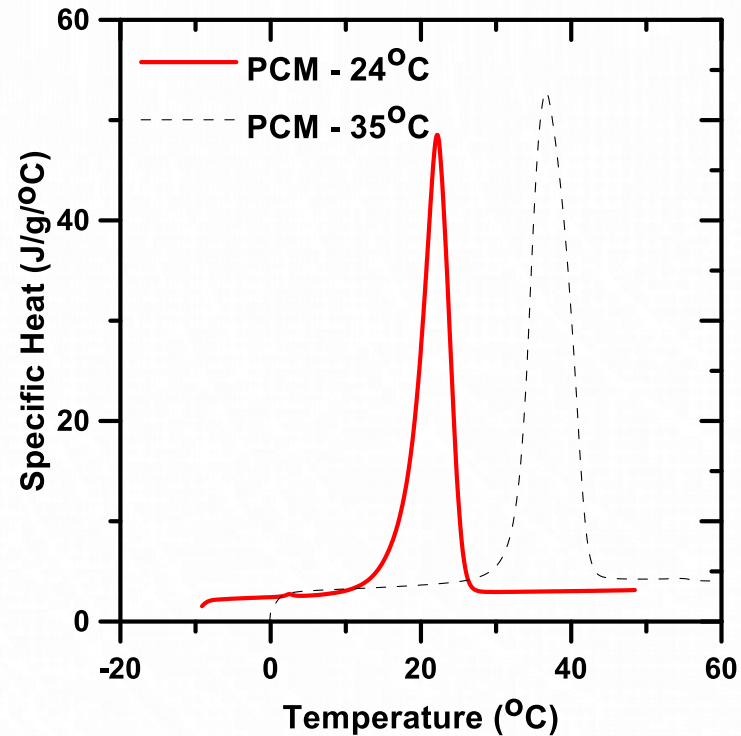
	PCM-M	PCM-E
Chemical Composition		
Paraffin wax (%)	69.50	90.22
Polymeric capsule (%)	20.70	7.67
Residue (%)	9.80	2.10
Core-to-shell ratio (mass-based)	3.4	11.8



# Thermal Energy Storage



# Characterization of Phase Change Materials

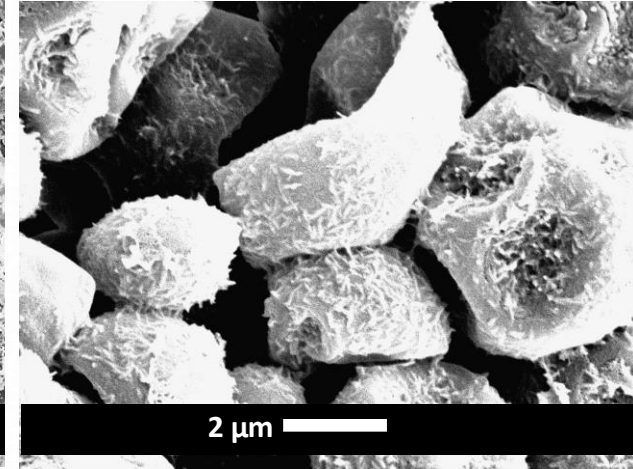
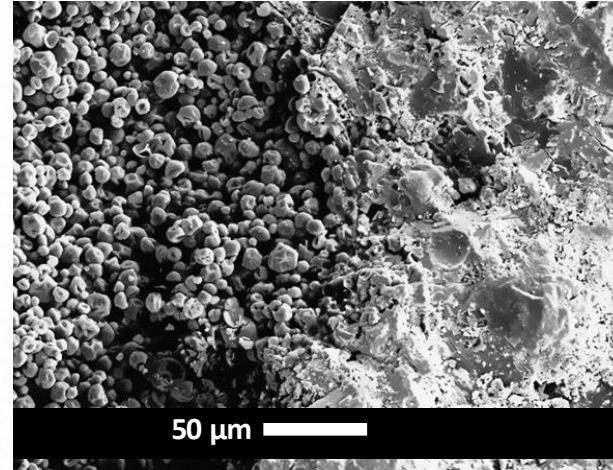
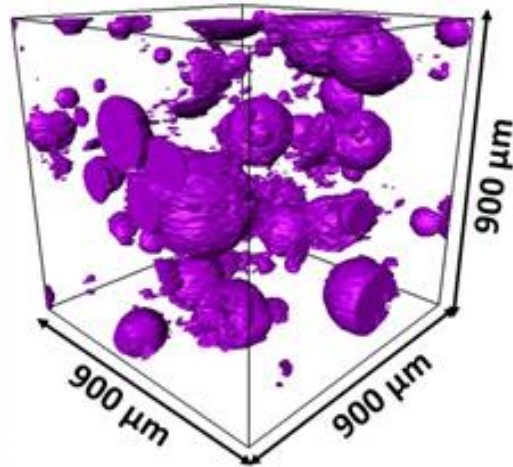


- PCMs absorb and release heat, which scales as a function of their dosage and enthalpy of phase change.
- During the phase transition, the PCM stores latent heat energy, helping to maintain a constant system temperature

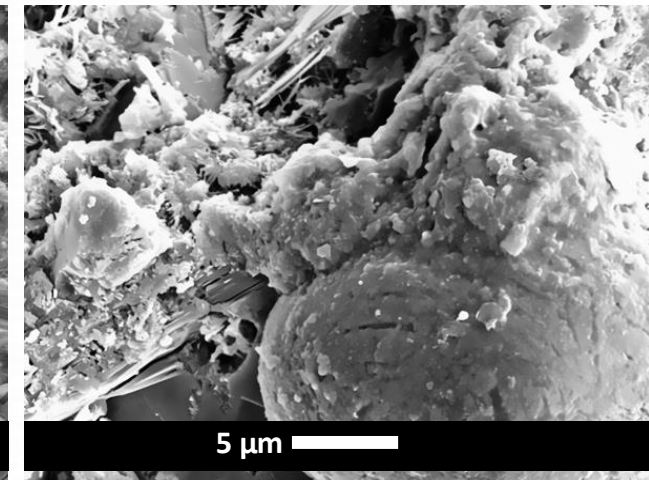
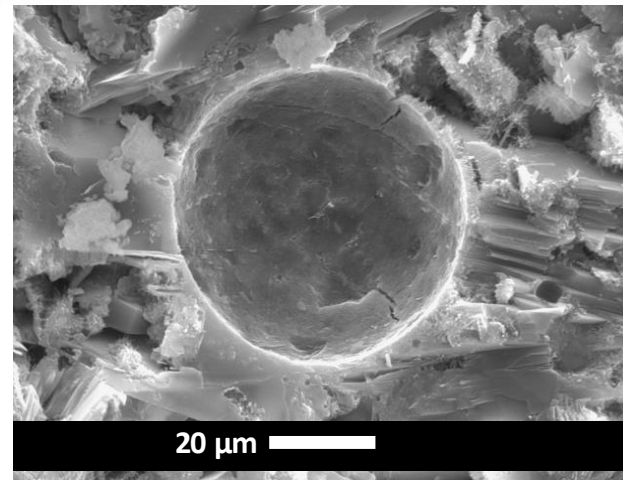
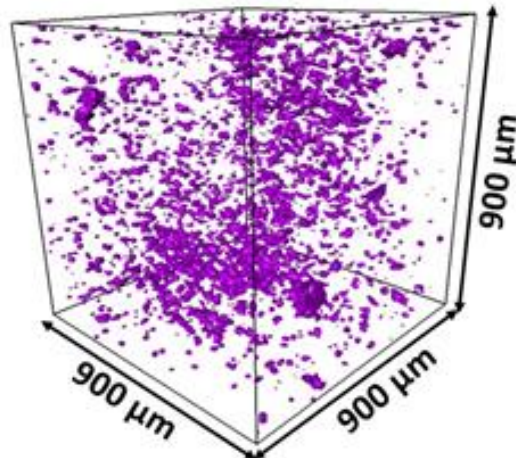


# Matrix Dispersion

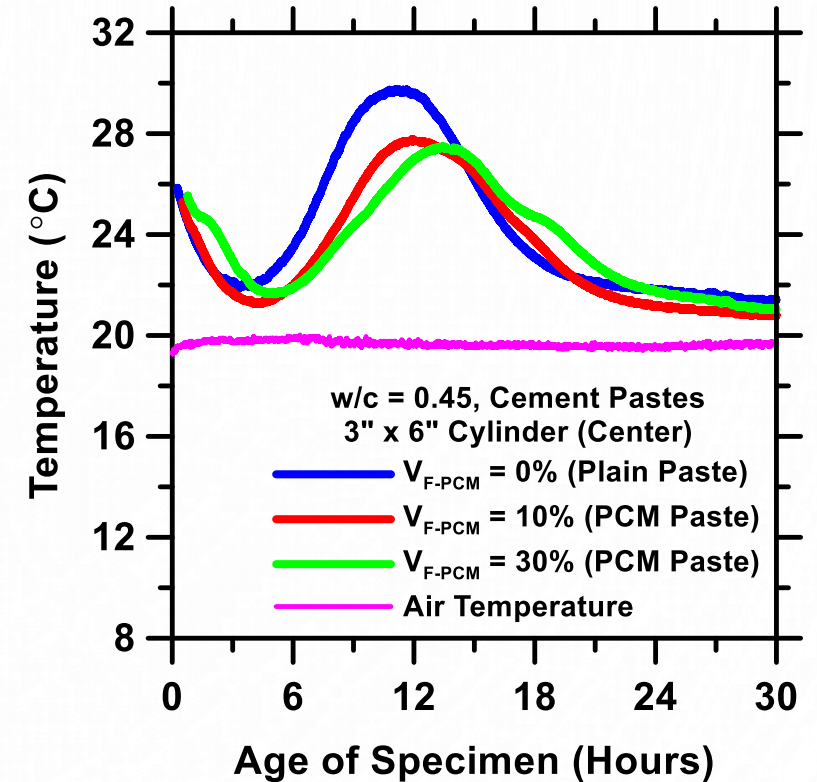
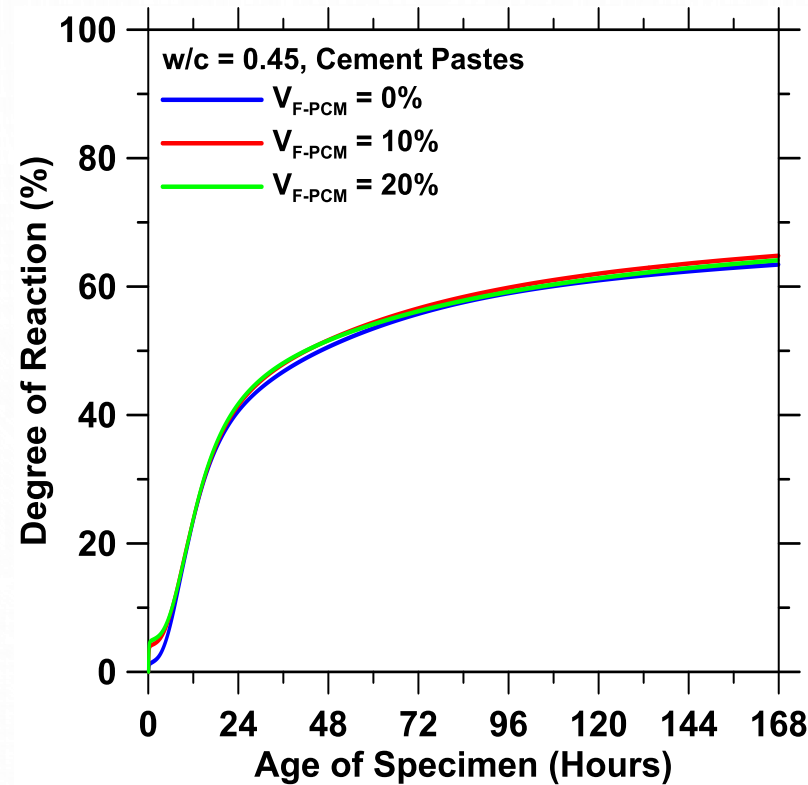
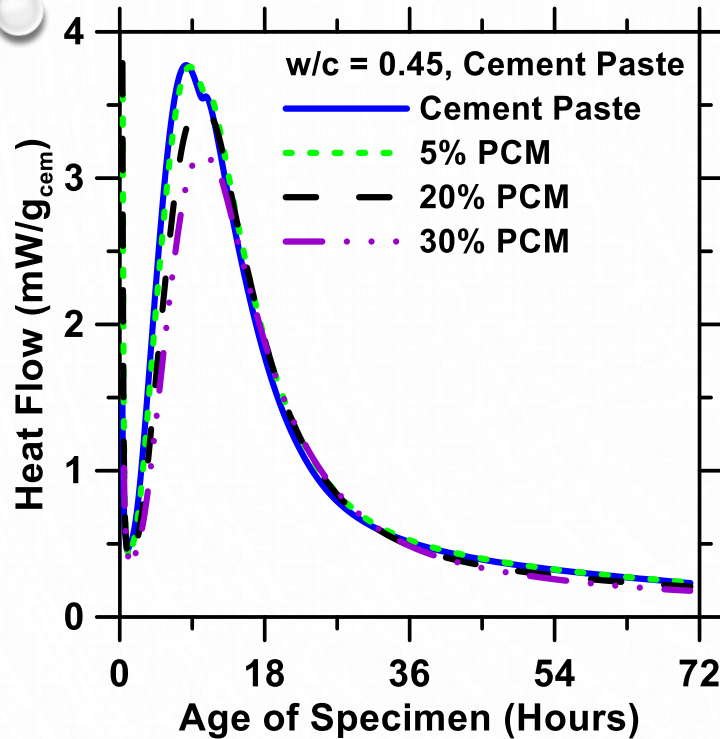
10% PCM-M by vol.



10% PCM-E by vol.



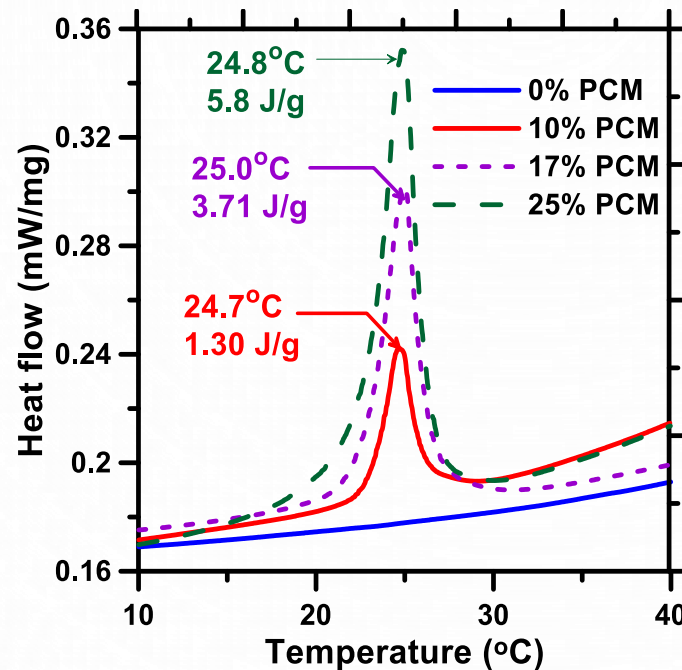
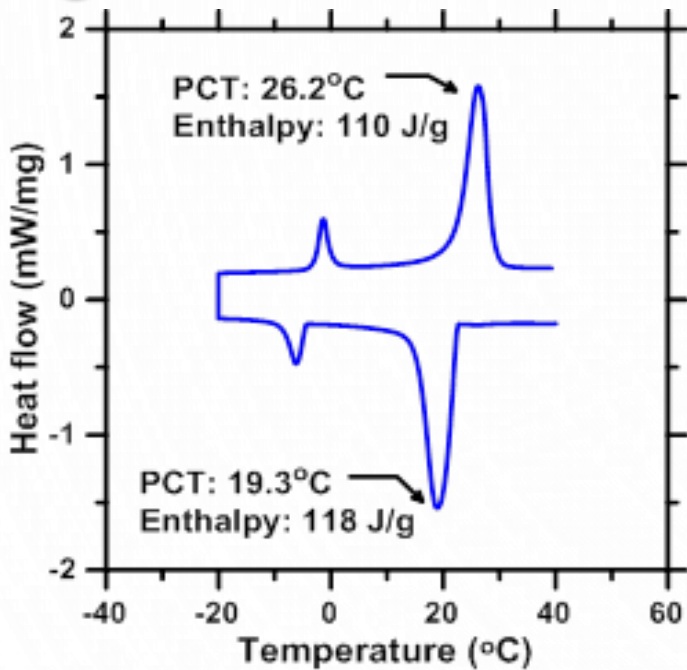
# Heat of hydration and temperature development



- Lower heat development – both isothermal and semi-adiabatic temperature rise
- Delaying time to peak temperature



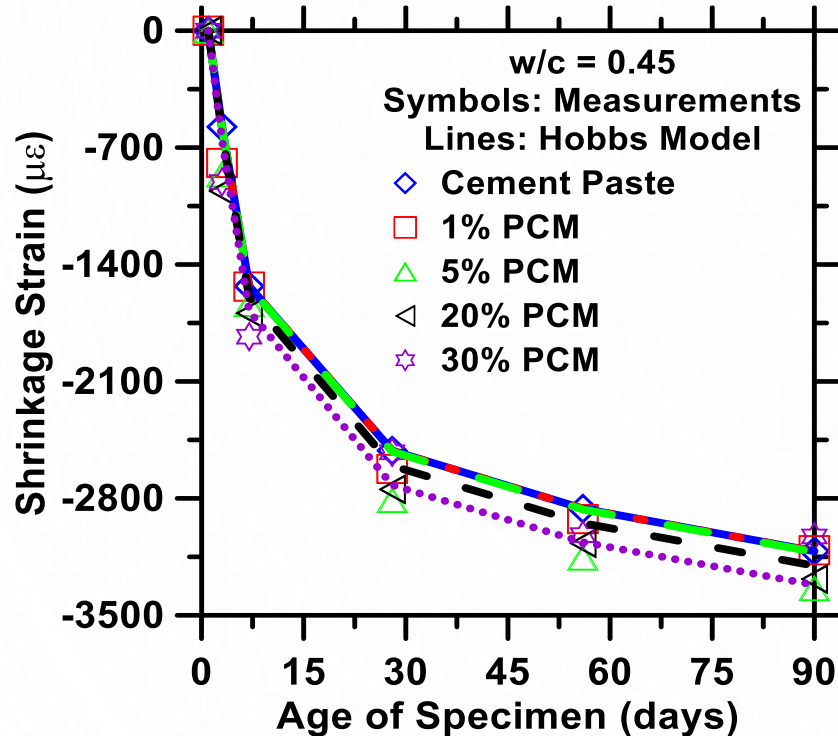
# Thermal Performance – heat flow



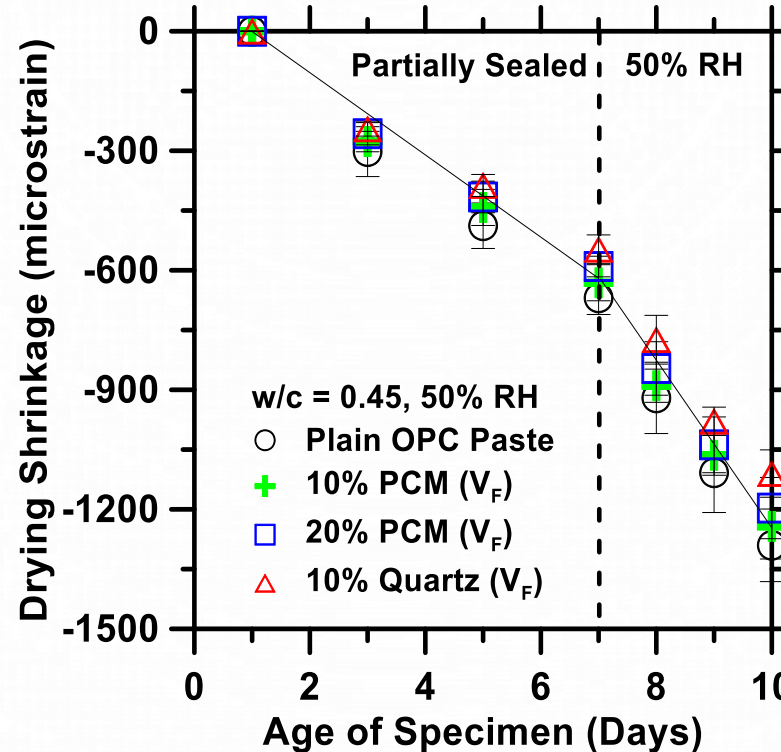
- The enthalpy of phase change determined as the area under the heat flow curve during the time the phase transition is active
- Onset temperature ( $T_{\text{onset}}$ ) corresponding to melting is 21.9°C and the completion temperature ( $T_{\text{completion}}$ ) is 29°C, with the endothermic peak noted at 26.2°C.

# Thermal Performance – thermal/shrinkage strains

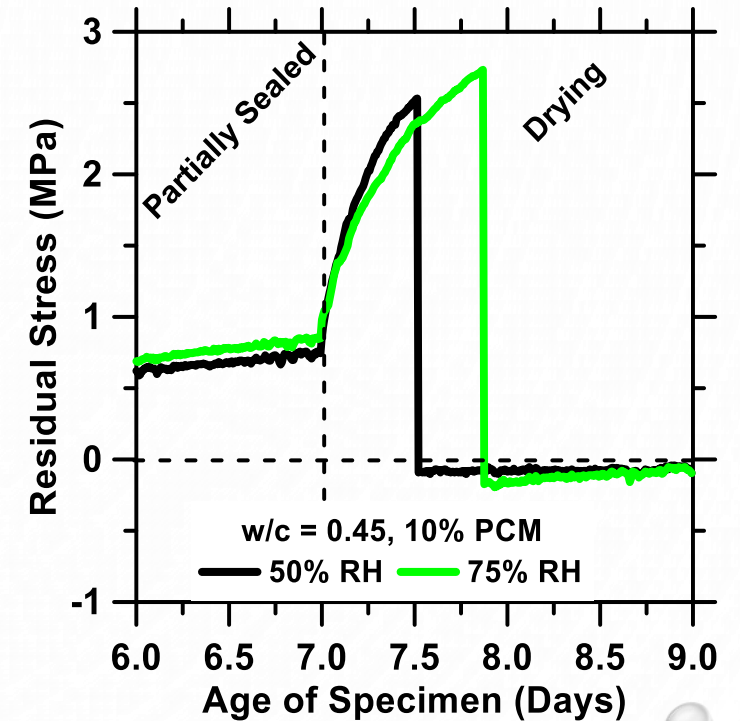
## Free shrinkage



## Partially sealed

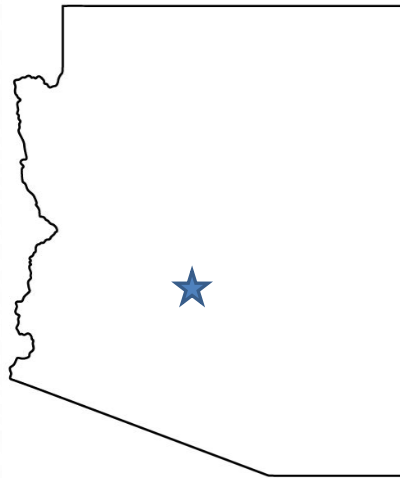


## Residual stress



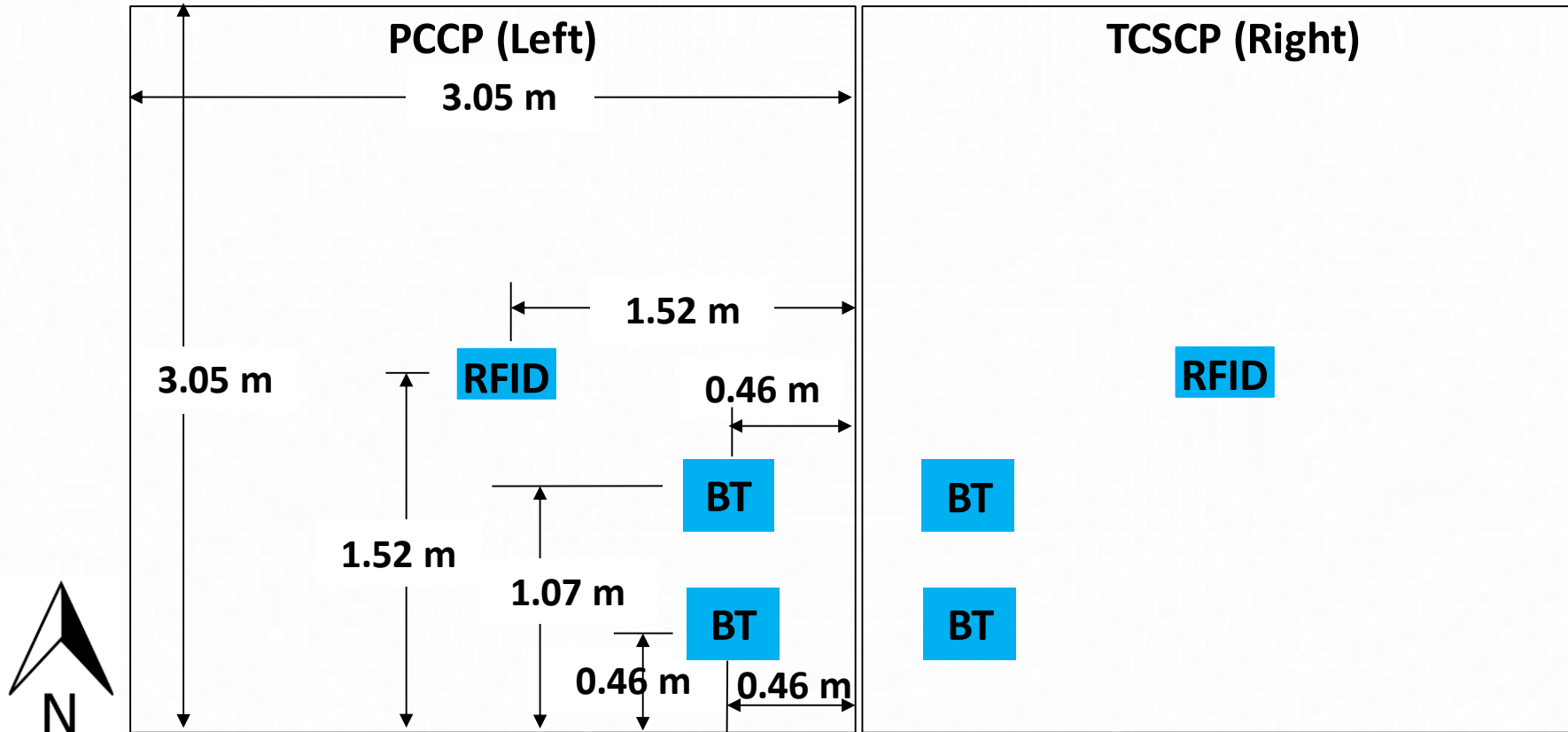
- No adverse influence on free strains; Delay in the age of cracking
- All the cementitious mixtures fail at tensile stresses substantially (around 50%) lower than the tensile strength of the material around 7 days

# Project Location

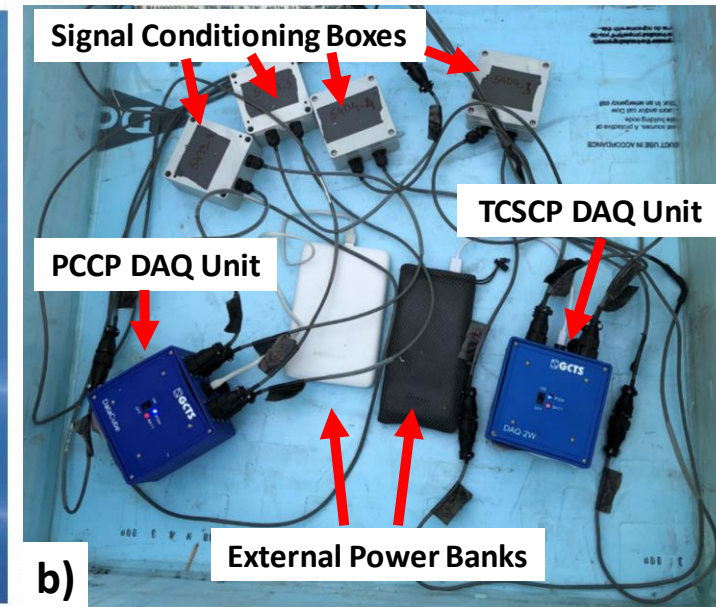
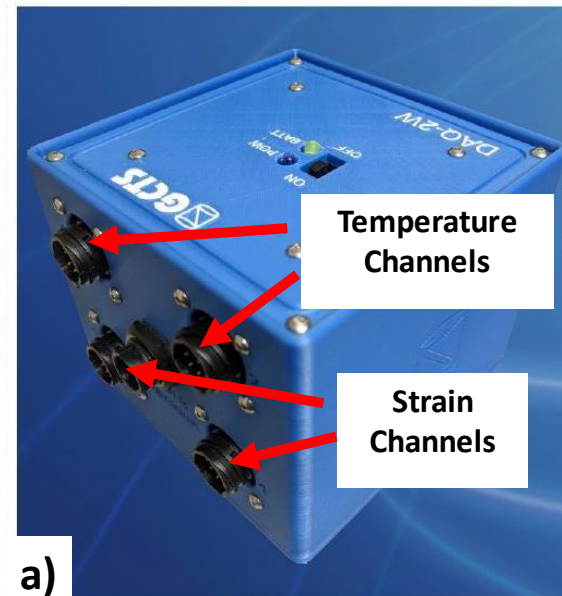
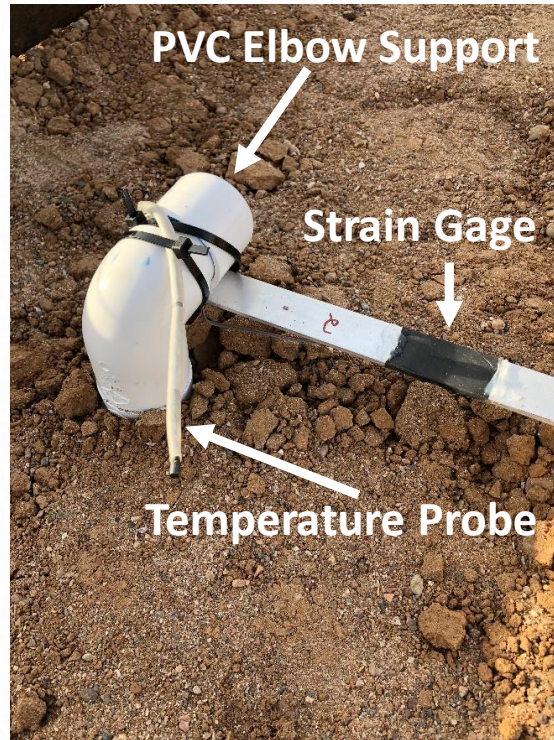




# Sensor Locations

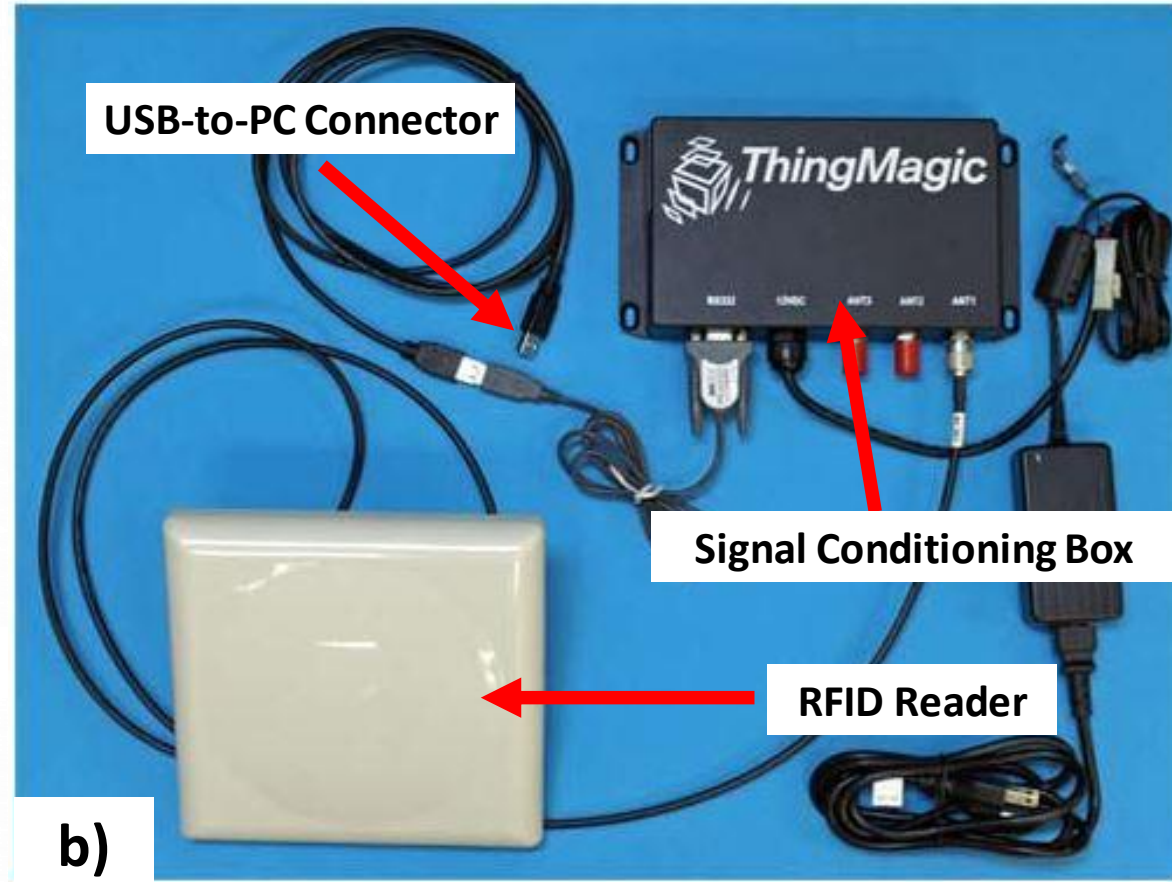
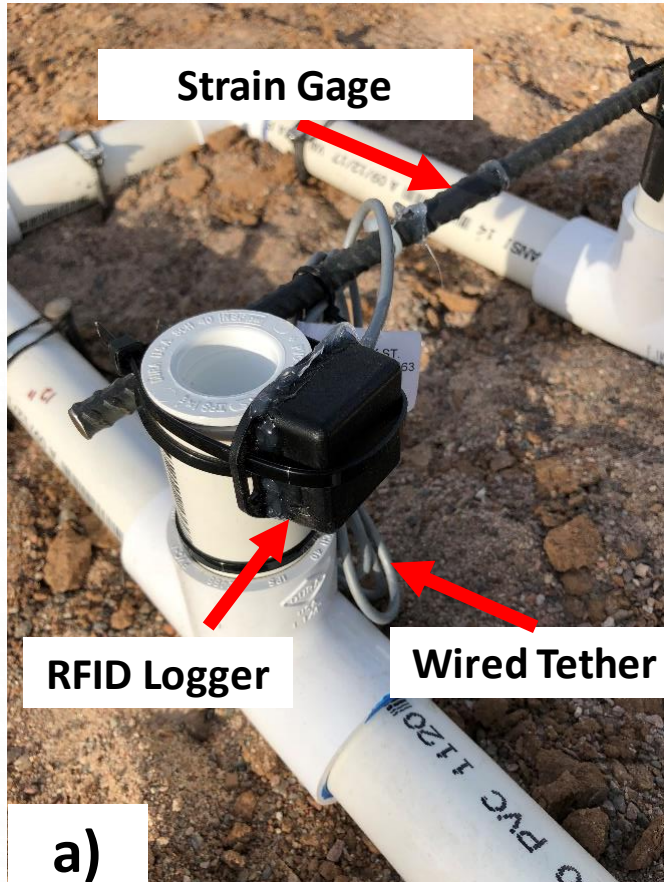


# Bluetooth Monitoring System



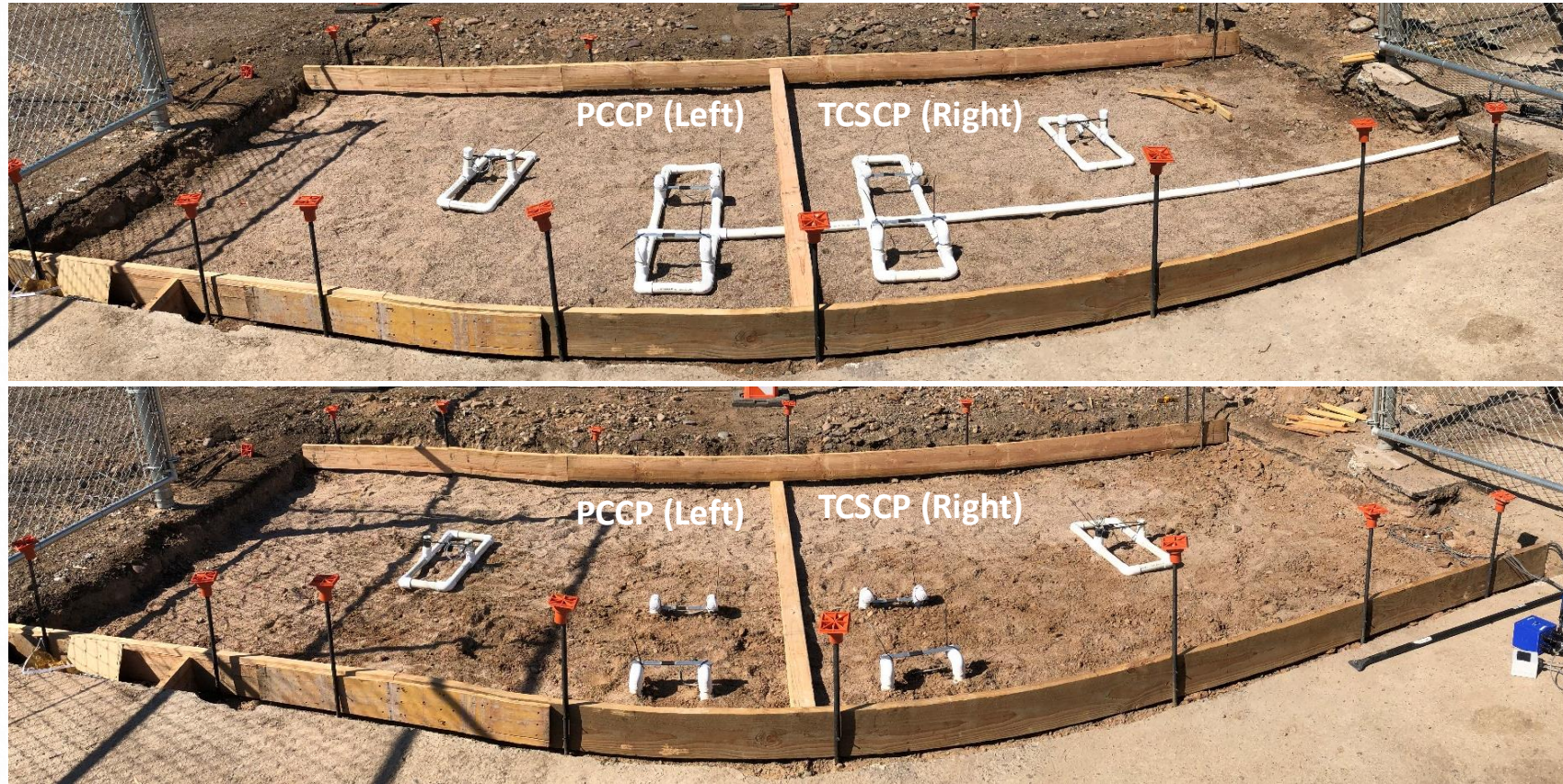


# RFID Monitoring System





# Sensor Locations





# PCM Dispersion



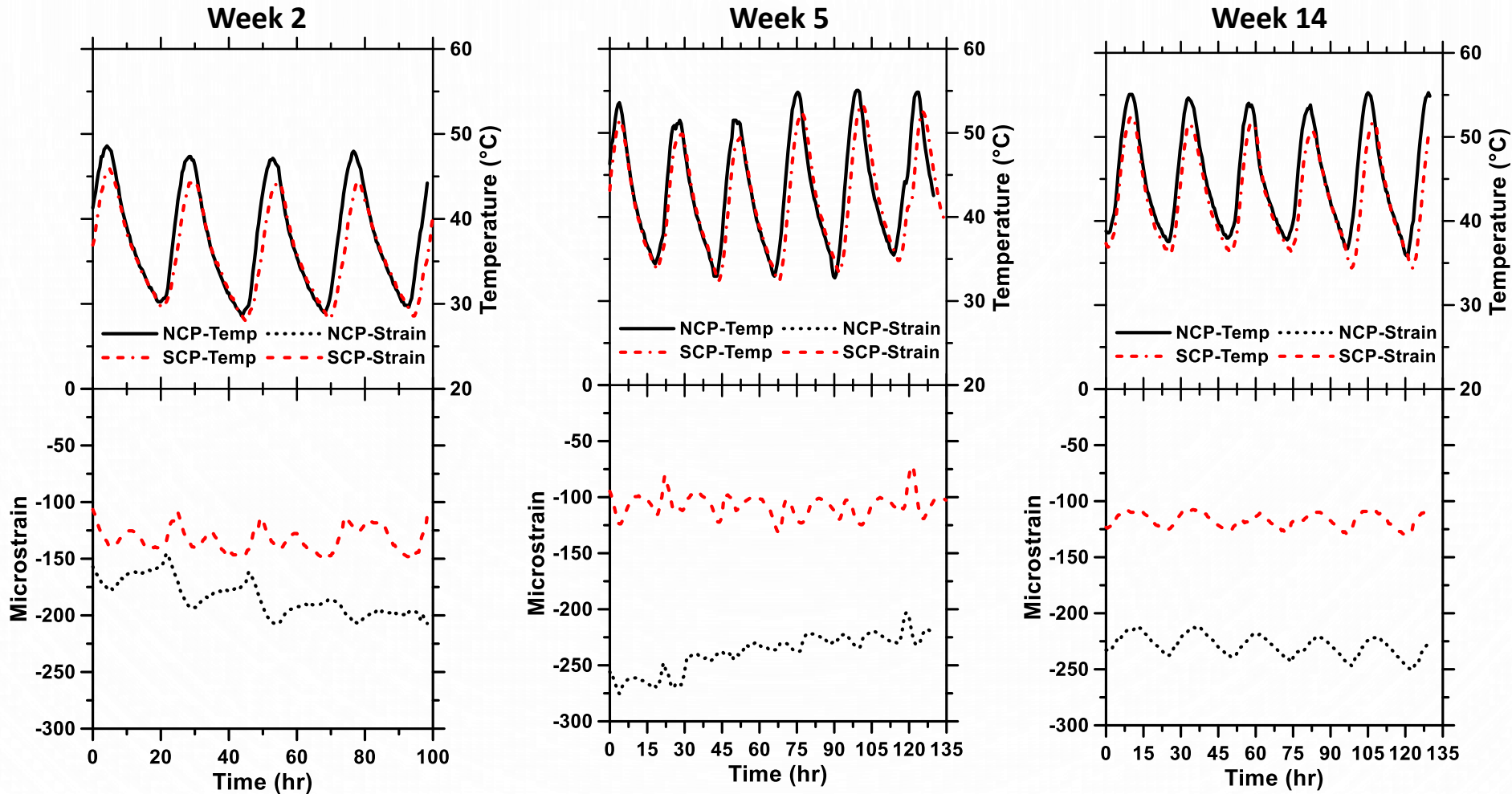


# Final PCCP and TCSCP Sections

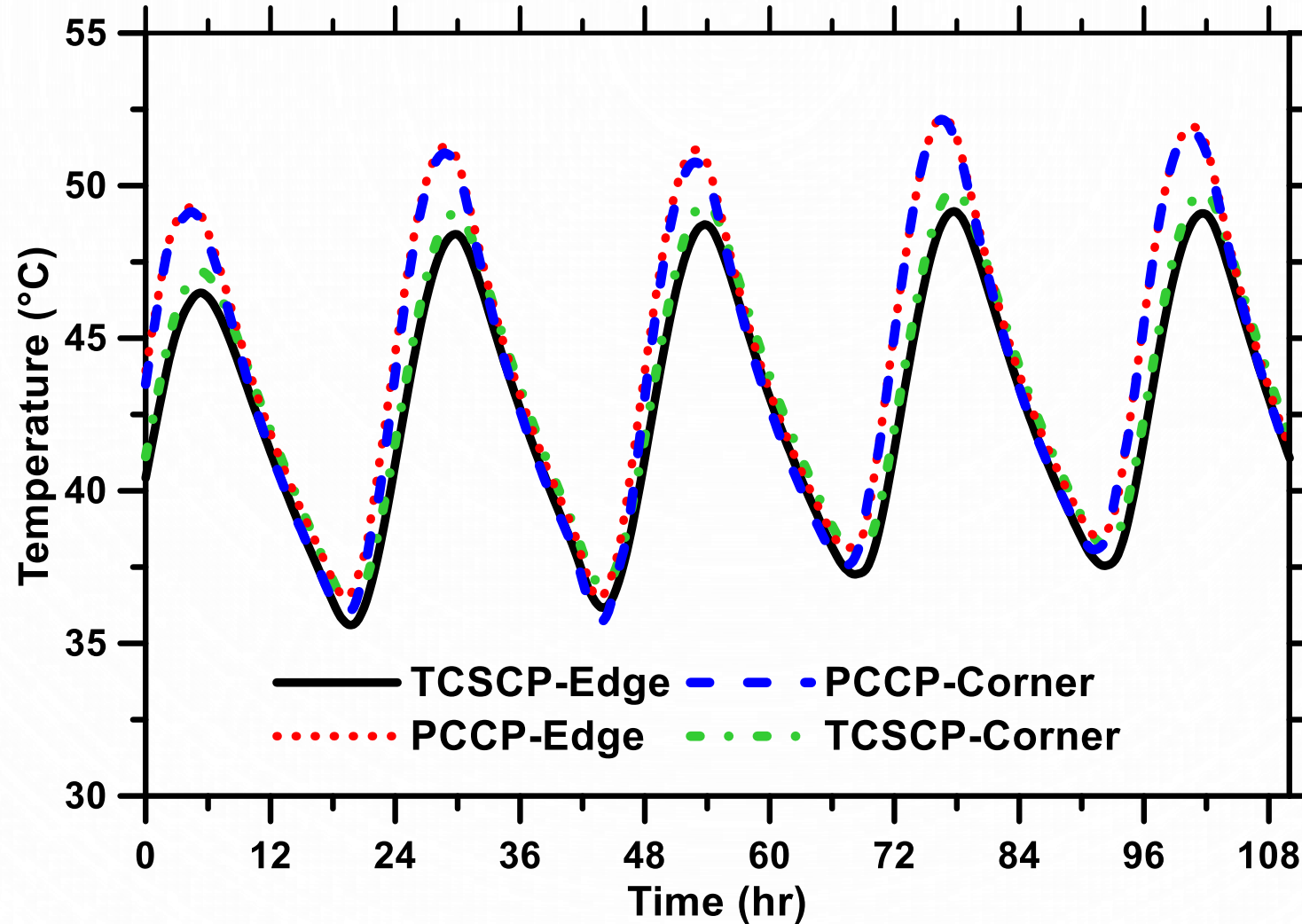




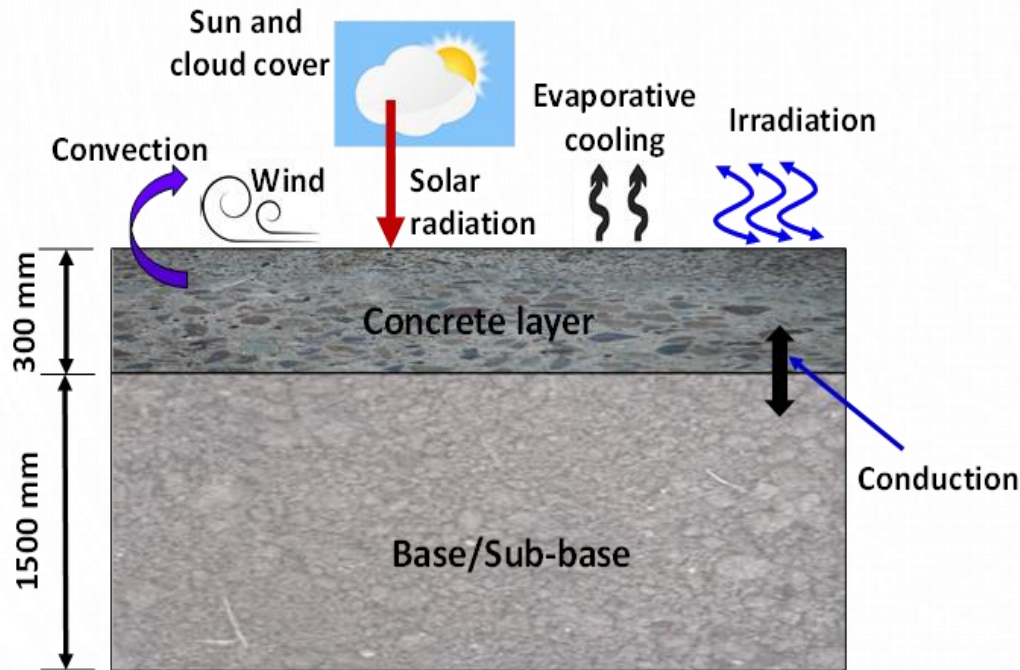
# RFID Temperature and Strain Development



# Temperature Development



# Numerical Modeling



$$-k_{comp} \frac{\partial T(0, t)}{\partial x} = h_0(T(0, t) - T_0) + \varepsilon\sigma(T_0^4 - T(0, t)^4) - \beta_s q_s$$

Convective heat transfer
Radiative heat transfer
Solar absorption

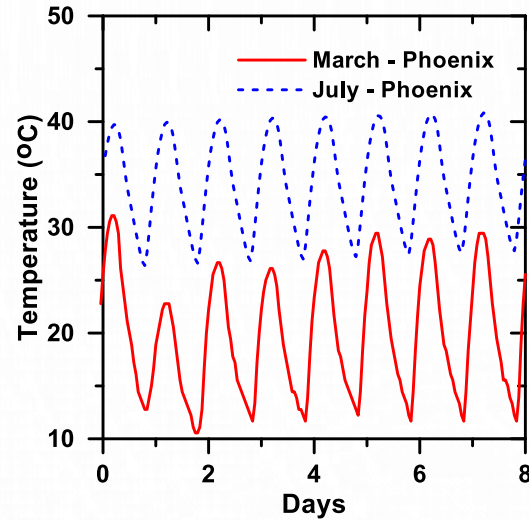
$$T(x, -L) = T_g$$

$$k_{comp} \frac{\partial T(L, t)}{\partial x} = k_{soil} \frac{\partial T_{soil}(L, t)}{\partial x}$$

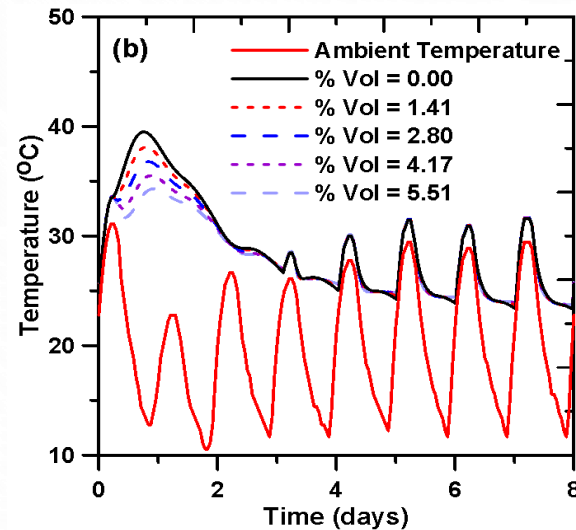
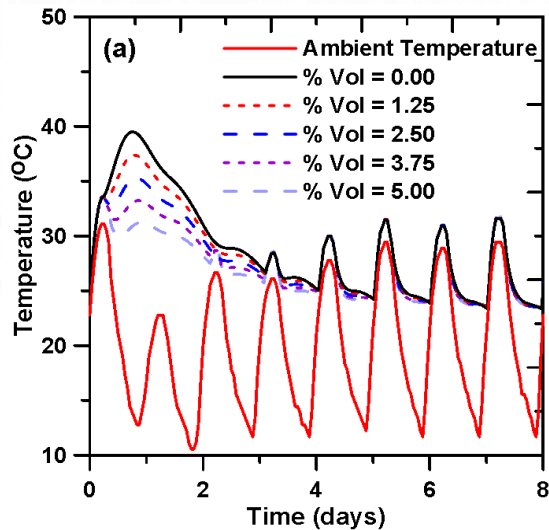
50 layers each were considered for the concrete pavement as well as the base/sub-base, which translated into layer thicknesses of 6 mm and 30 mm respectively for these components



# Temperature profile



- Maximum section temperatures when PCM replaces cement or sand by a certain volume

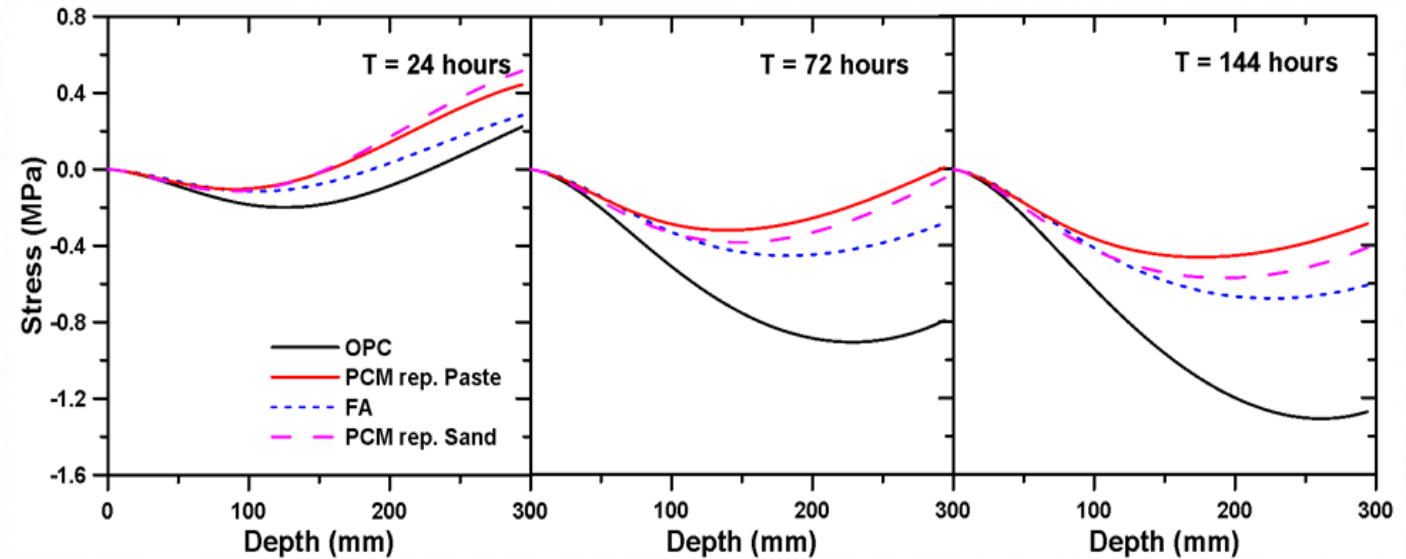


# Stress development

$$\Delta T_{axial} = \frac{\sum_{z=0}^h [(T_{z,current} - T_{z,final-set}) \Delta z]}{h}$$

$$\varepsilon_{axial}(t) = \alpha_{comp}(t) \cdot \Delta T_{axial}(t)$$

$$\sigma_{axial}(t) = E(t) \cdot \varepsilon_{axial}(t)$$

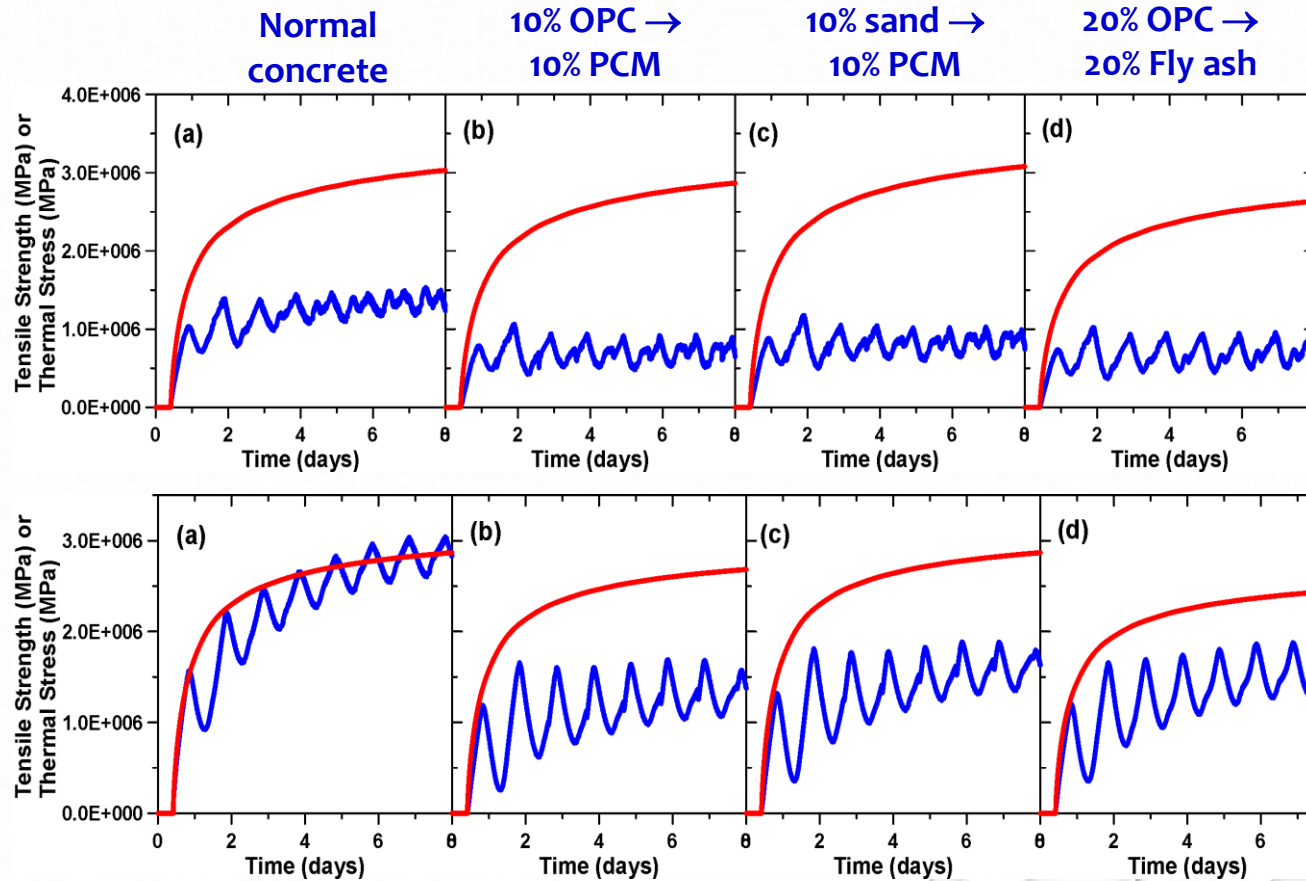


$$\Delta T_{curl} = (T_{top,current} - T_{bottom,current}) - (T_{top,final-set} - T_{bottom,final-set})$$

$$\sigma_{curl} = \frac{\alpha_{comp} \cdot \Delta T_{curl} \cdot E}{2(1 - \nu^2)} (C_{max} + \nu C_{min})$$

# Thermal stress and cracking potential

$$\sigma_{critical} = MAX_{tensile} \begin{cases} \varepsilon_{axial,top} \times R_F(top) \times E + \sigma_{curl,top} \\ \varepsilon_{axial,z} \times R_F(z) \times E \\ \varepsilon_{axial,bottom} \times R_F(bottom) \times E + \sigma_{curl,bottom} \\ 0 \end{cases}$$



Phoenix - March

Phoenix - July



# Conclusions

- Laboratory and field studies on the use of phase change materials in concrete
- Thermal stress/strain mitigation and crack control
- RFID strain sensing for quality control and prediction
- Experiments, models, and field studies to establish the beneficial attributes of a novel technology

# Acknowledgements



U.S. DEPARTMENT OF  
**ENERGY**

