

Implementation of UHPC Decked I-Beam in Ontario, Canada

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THE WORLD'S GATHERING PLACE FOR ADVANCING CONCRETE



Presentation Outline

- Background
- Design
- Fabrication
- Testing

Background

- Part of PCI study on the Implementation of UHPC Structural Elements for Bridges and Buildings (PI: eConstruct USA).

e.construct
Structural Engineering Consultants

Implementation of Ultra-High Performance Concrete in Long-Span Precast Prestensioned Elements for Concrete Buildings and Bridges

Phase II Report
September 15, 2021

PCI

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Participating Research Agencies:



Participating Precast Concrete Companies:

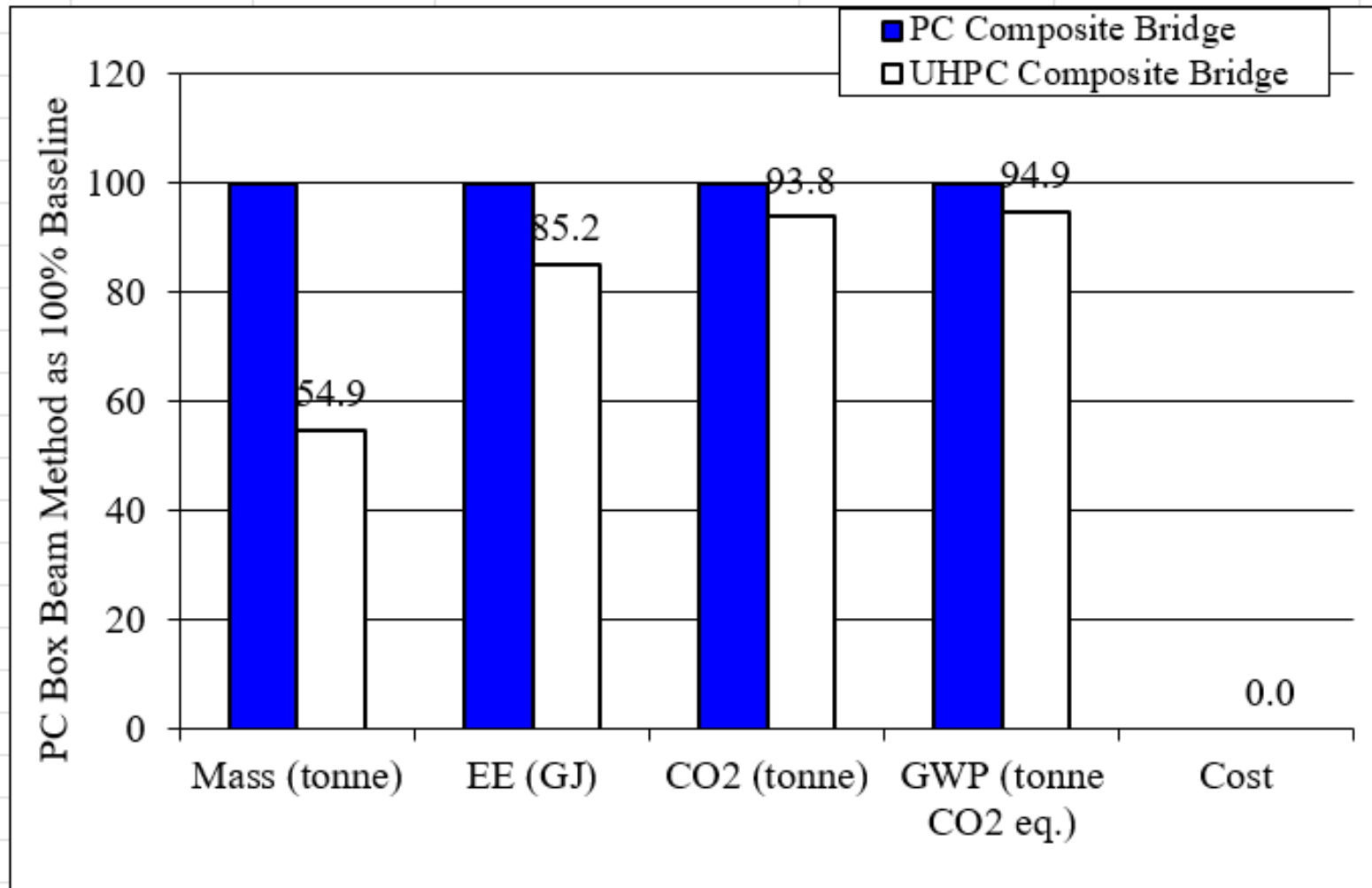


Participating Steel Formwork Companies:



Environmental Impact: PC Box Beam vs UHPC

NUDIB



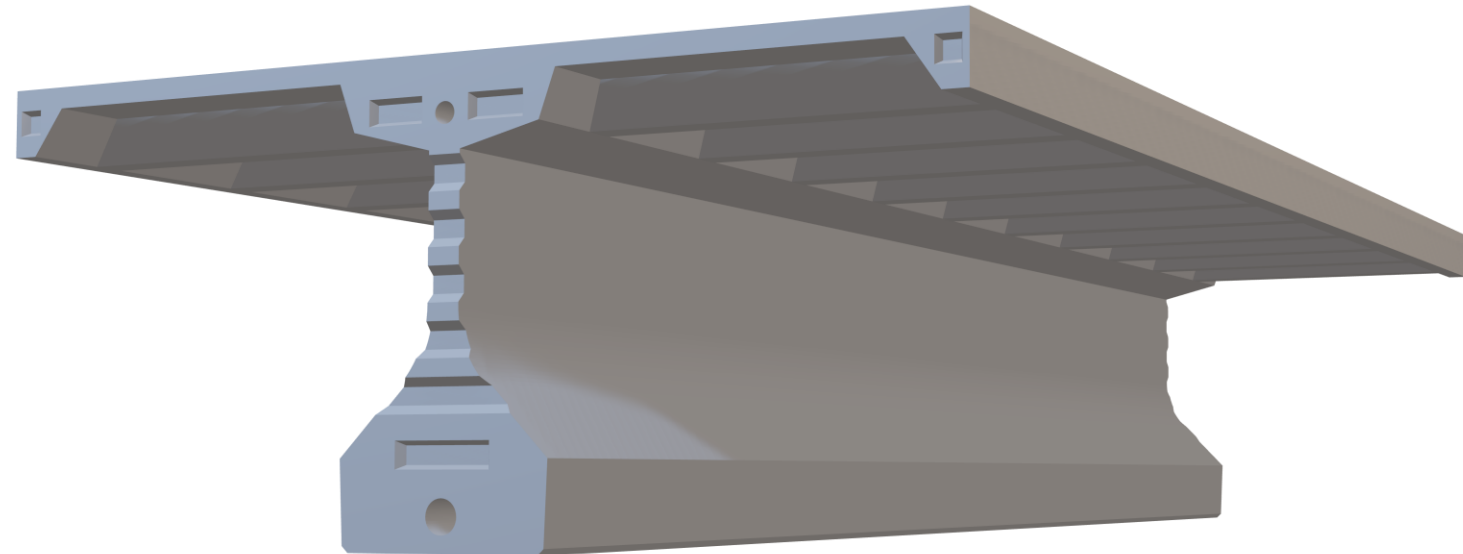
Based on Voo & Foster in 2020, which showed an expected Service life for the UHPC bridge girders Of 340 years.

UHPC Decked I-Beam Development

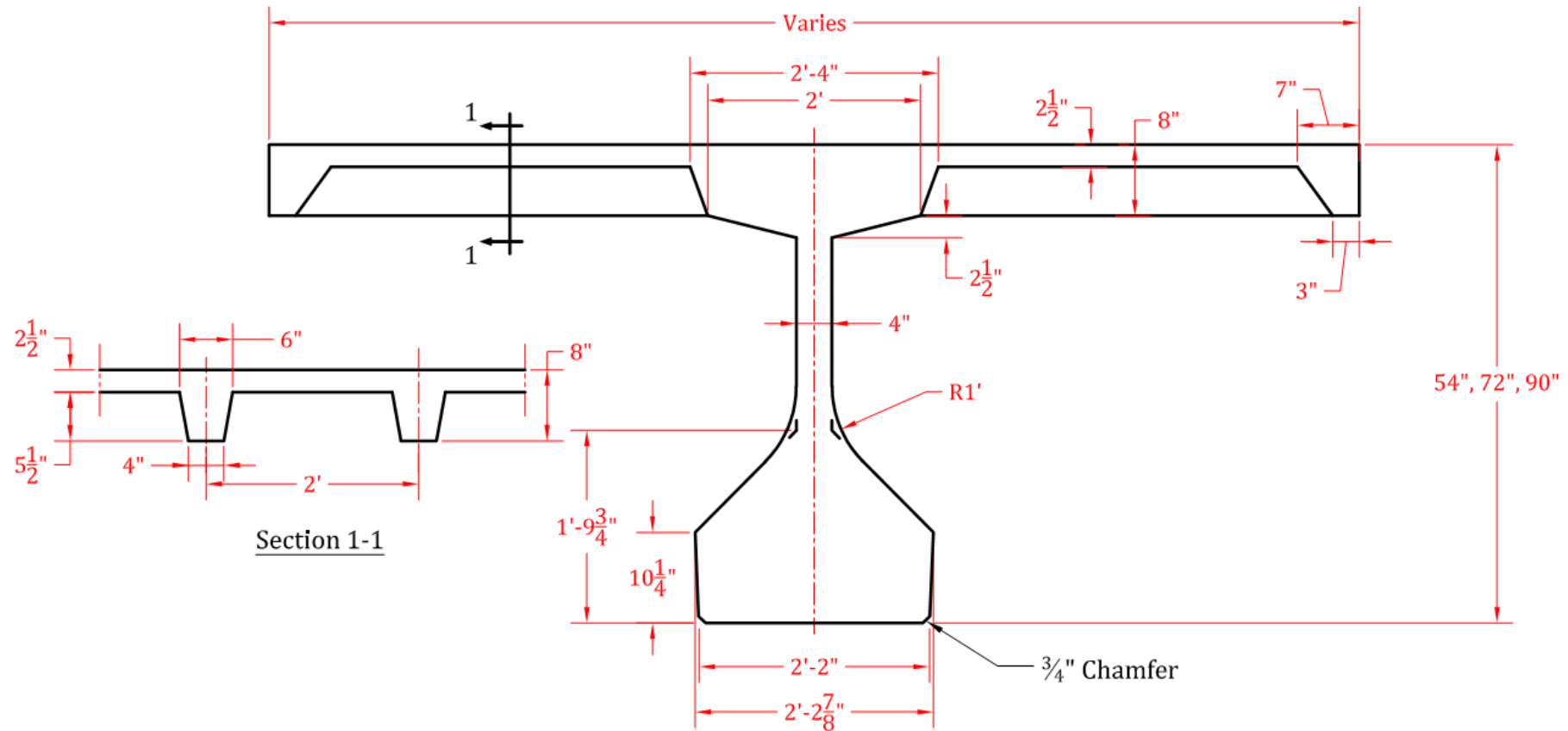
- The girder cross section shape was developed by e.construct USA, (Dr. Maher Tadros, Mr. Adam Sevenker, Dr. Mostafa Aboelkhier)
- It was optimized to meet the following criteria
 - Accelerated bridge construction (ABC), using the least number of precast pieces and minimizing cast-in-place usage.
 - Superior durability against freeze-thaw cycling and saltwater contamination
 - Superior tensile strength
 - Least concrete volume while maintaining satisfactory stiffness

Three-Dimensional View of Beam

Product can be pretensioned, or segmentally post-tensioned.



Cross Section Dimensions

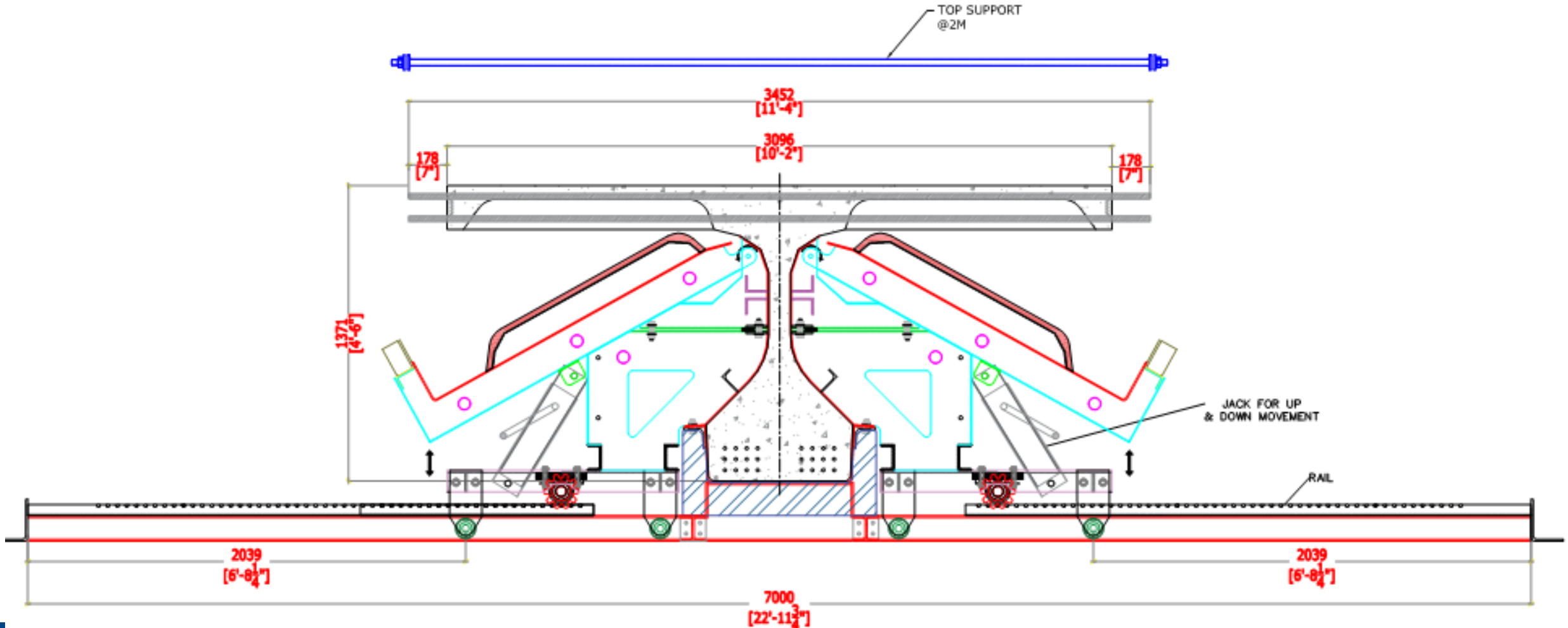


Challenges of Use of Decked I-Beam with Ribbed Top Flange

- Forms are more complicated than with conventional I-beams. But this is a one-time cost.
- Crown requires sloping beams at 2%
- Flange to flange connection requires that the flange edges be totally aligned. Possible to use a clamping mechanism.
- Full 8 in. (200 mm) thickness may be needed for ends of skew bridges. The forms must be designed to allow for void form removal.
- UHPC beams connected at the site with a UHPC CIP closure strip produce a challenging vertical cold joint.

Forming Concept

Courtesy Kessab Steel, Dubai, UAE



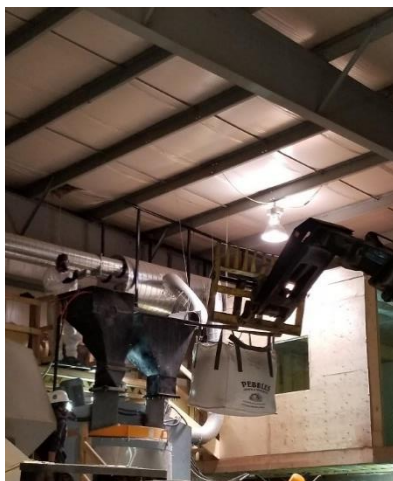
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Girder Form and Reinforcement

- The formwork materials included steel, plastic, and roughened Styrofoam
- Formwork was pre-heated to a minimum of 10 °C (50 °F)
- Temperature sensors were attached to some bars



Dura® UHPC Batching



- High shear Skako mixer (0.4 m³ (0.5 yd³)) and two pan mixers (0.15 m³ (0.2 yd³))
- Batch ready every **20-25** minutes
- Ice and workability admixture were used to maintain flowability

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Batching Sequence

- Add dry powder, mix, then water and chemicals – mix
- Add fibers after flowability is assured
- Discharge using traditional buckets
- Stack batches and deliver to mold

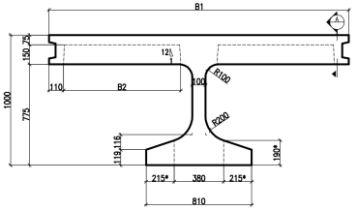


Concrete Placement

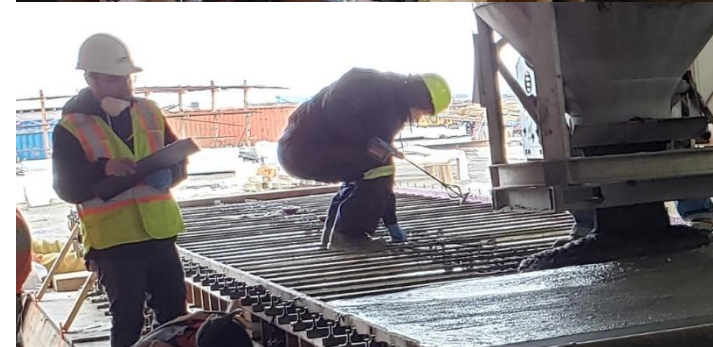


- A special chute was built to convey the materials from the bucket to the lower bell and the web
- While the material was discharged, vibration was externally applied from both sides of the web, to aid flow
- The exposed UHPC surface was immediately covered with wet burlap

Placement-Continued



- After concrete filled the web and bottom flange, pouring of the deck started
- Vibration for a short period was applied under each waffle to ensure the entire waffle section and key were filled
- Excess material was levelled with a spiked roller
- While waiting for the next bucket, the exposed surface was agitated using a paint drill bit



Deck Finish

- Fine ribbed rubber matting sheets were placed on the surface after levelling
- Then a heavy steel roller was moved back and forth on top of the rubber matting, to force the concrete to fill the rubber ribs
- The rubber matting ribs were oriented parallel to the traffic direction

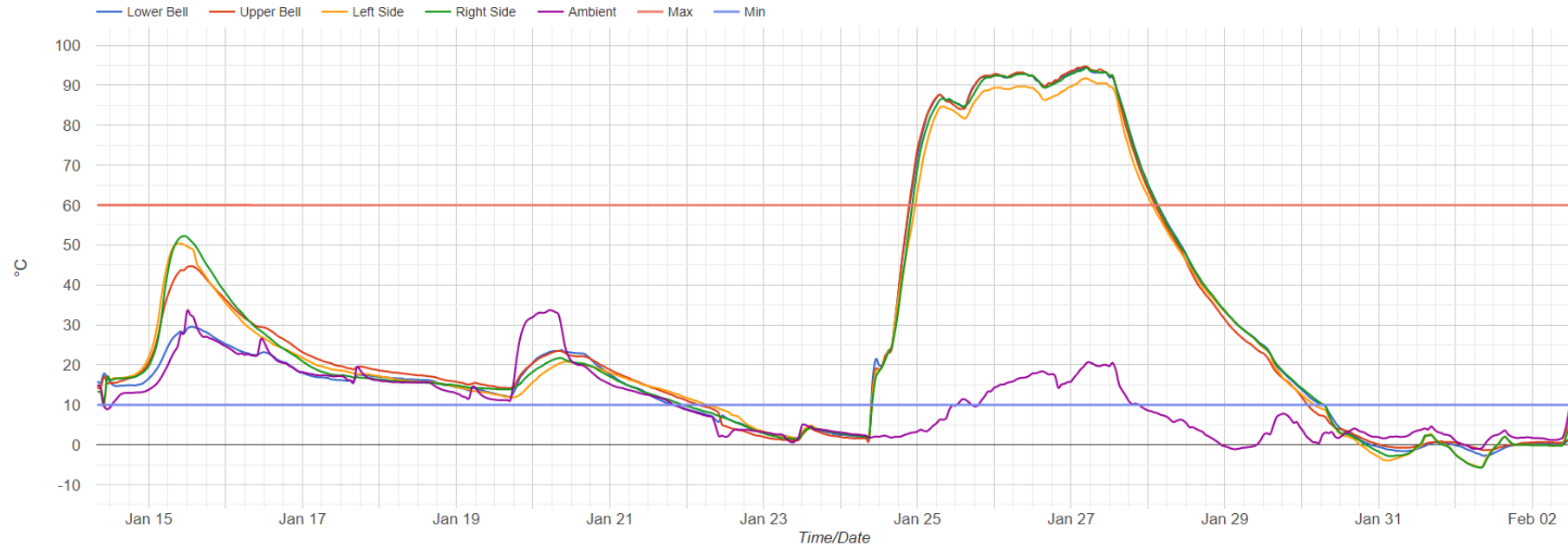


Steam Curing



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Curing & Post-Curing Temperature Profile



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Quality Control

- After thermal curing, the DIB girder was covered with an isolated tarp
- A total of 144 cylinders and 15 prisms were molded from 8 different batches
- Specimens were generally taken from a combination of two or more batches
- The cylinders and prisms were left to cure with the product
- Testing at local Ontario CCIL certified lab, WJE Chicago and NCSU



Testing at North Carolina State University

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Girder Shipped on February 21, 2020

FACCA Inc. in Ruscom, Ontario, Canada



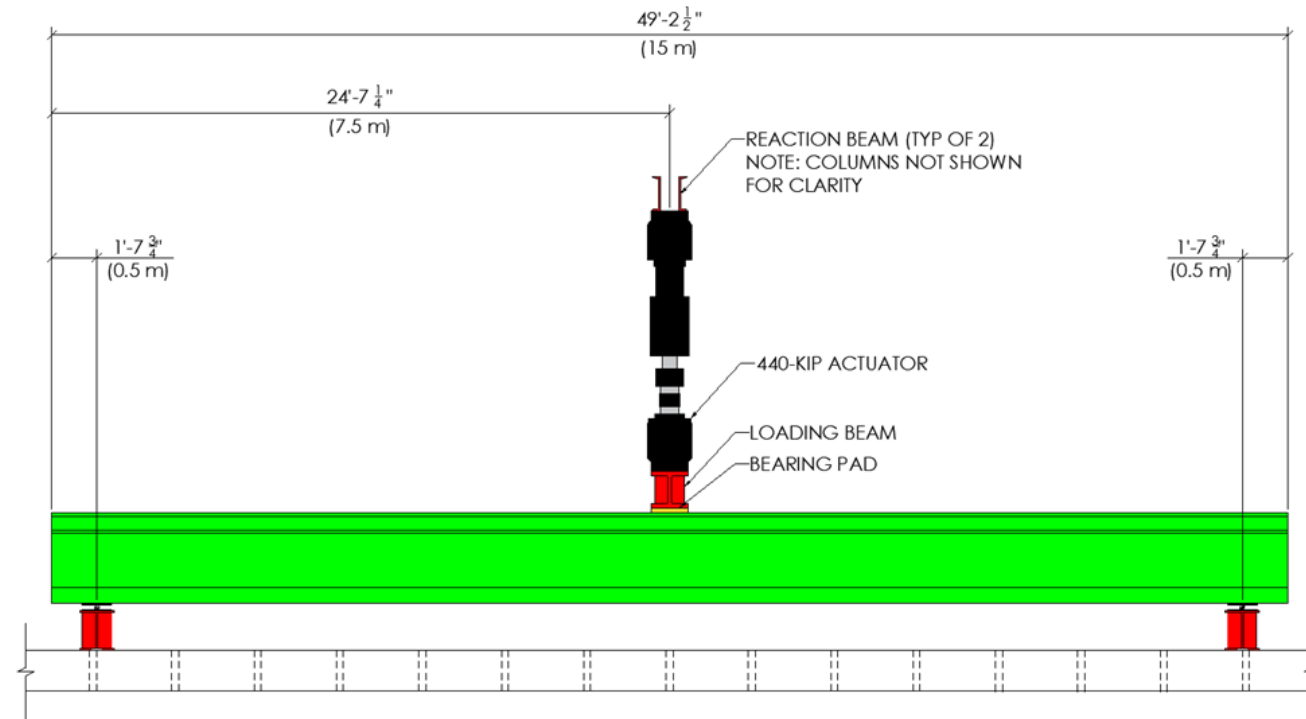
NC State Univ. in North Carolina,
USA



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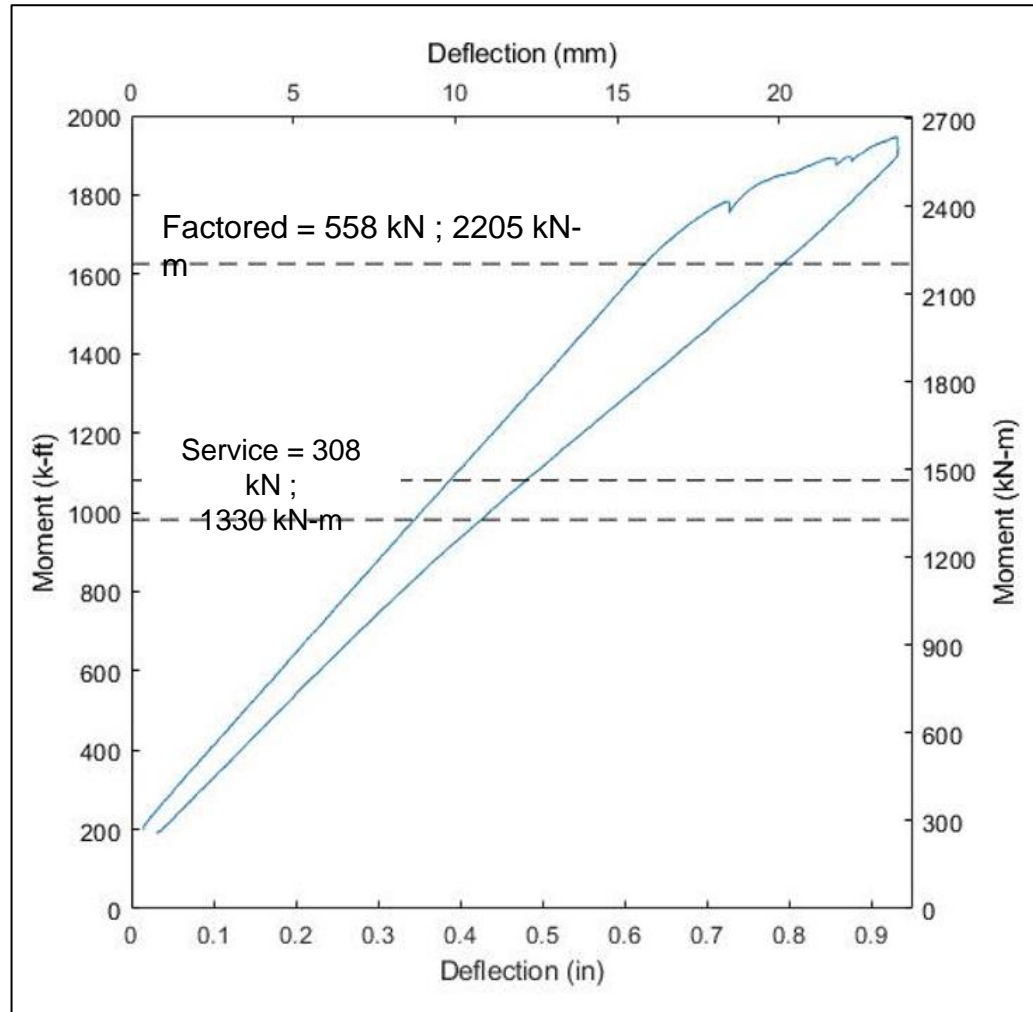
Flexure Testing

Goal: Verify flexural behavior. Stop the test prior to excessive damage so the beam ends can be tested in shear.



Flexure Testing

Source:
NCSU



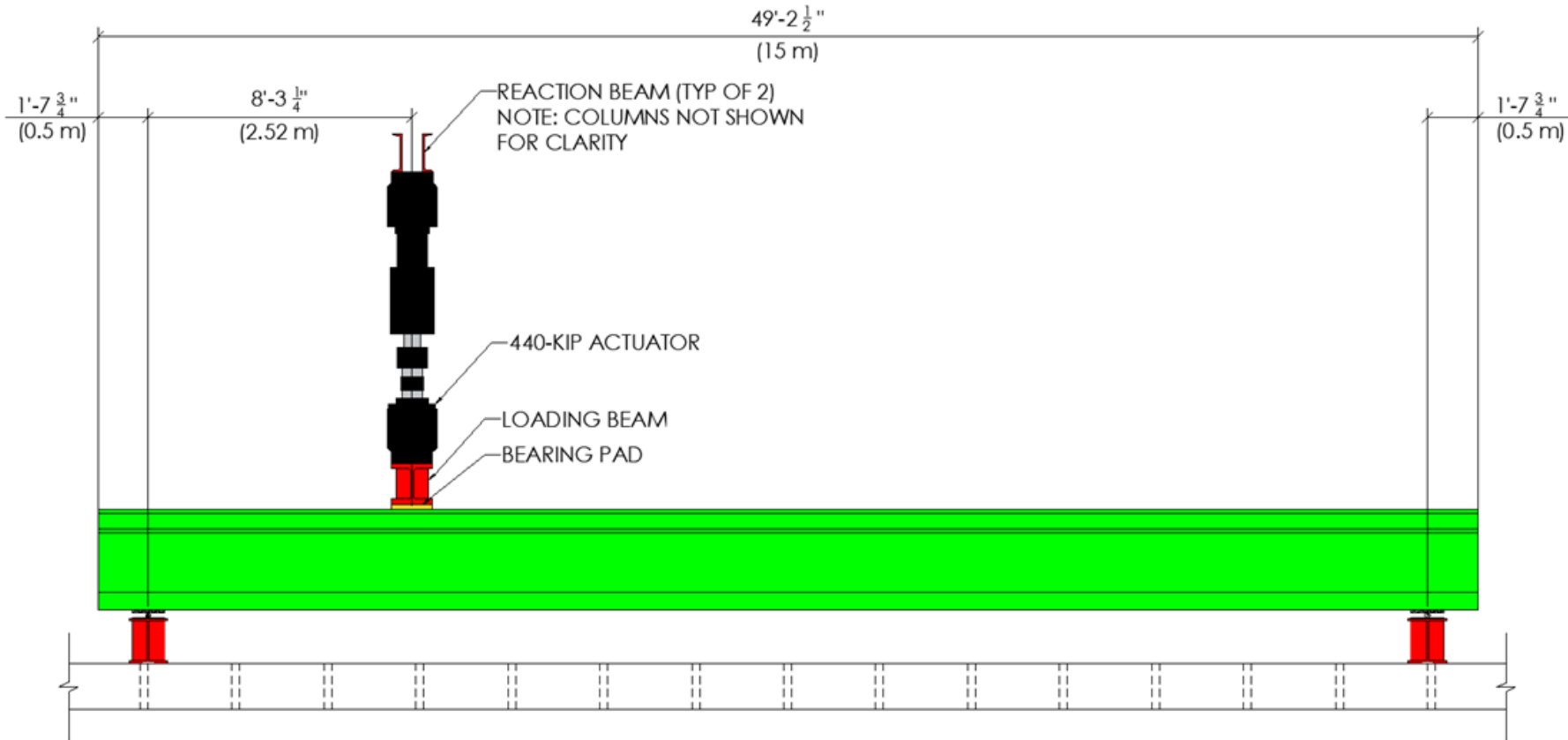
Service = 308 kN (69 kips)
Factored = 558 kN (125 kips)
Theoretical Cracking = 591 kN
Experimental Cracking = 578 kN

Test was stopped and unloaded after reaching 689 kN (155 kips), with no visible damage.

Theoretical Capacity = 875 kN (197 kips)



Shear Testing



Goal: Verify behavior to failure on both sides of the beam.

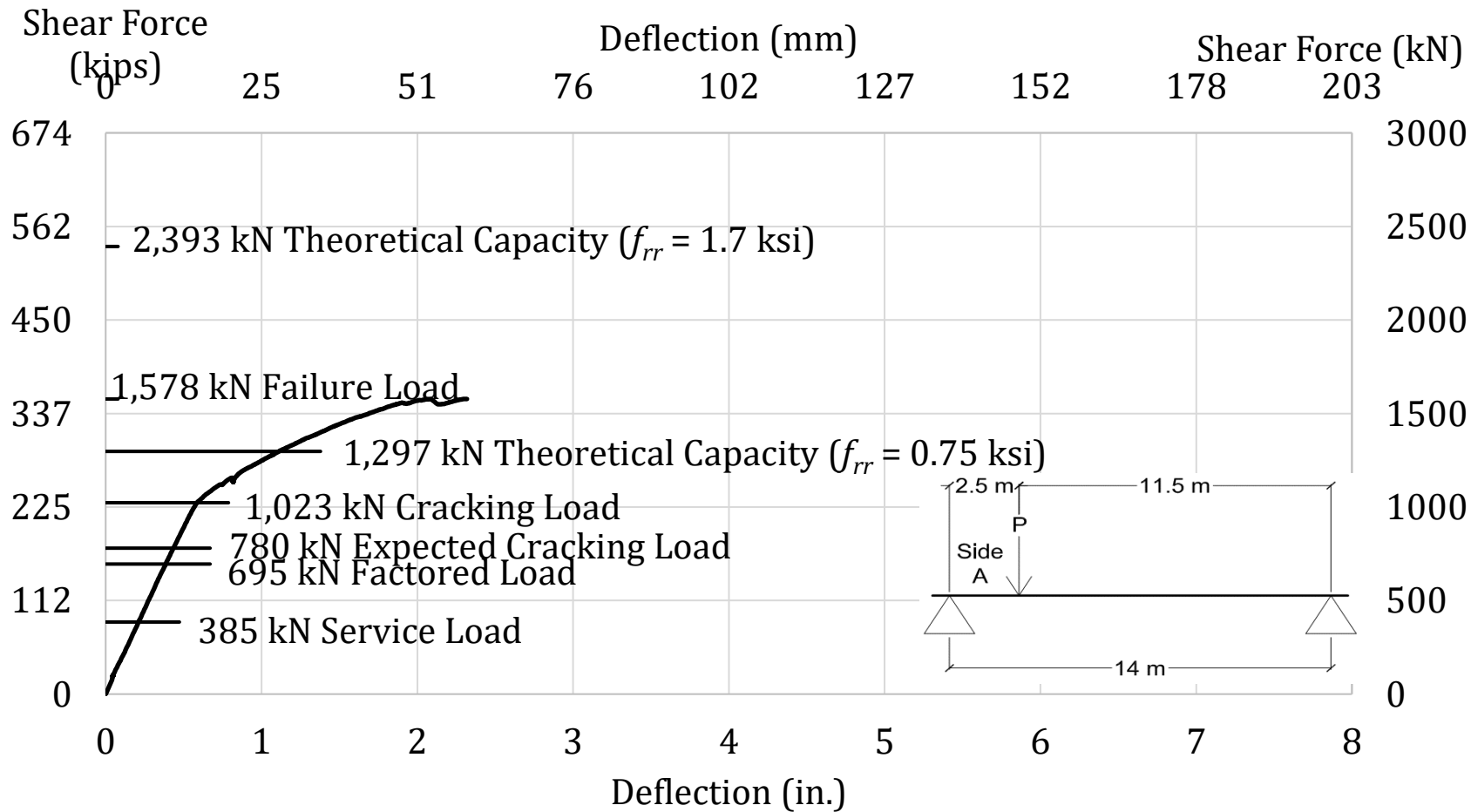
One side with steel stirrups (US #5 bar at 250mm)

Other side with no stirrups (UHPC only).



Shear Testing – Side with No Stirrups

Source:
NCSU



Shear Testing – Side with No Stirrups

Source:
NCSU



Steel fibers bridge the crack at peak load

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Shear Testing – Side with No Stirrups (Video)



Source:
NCSU

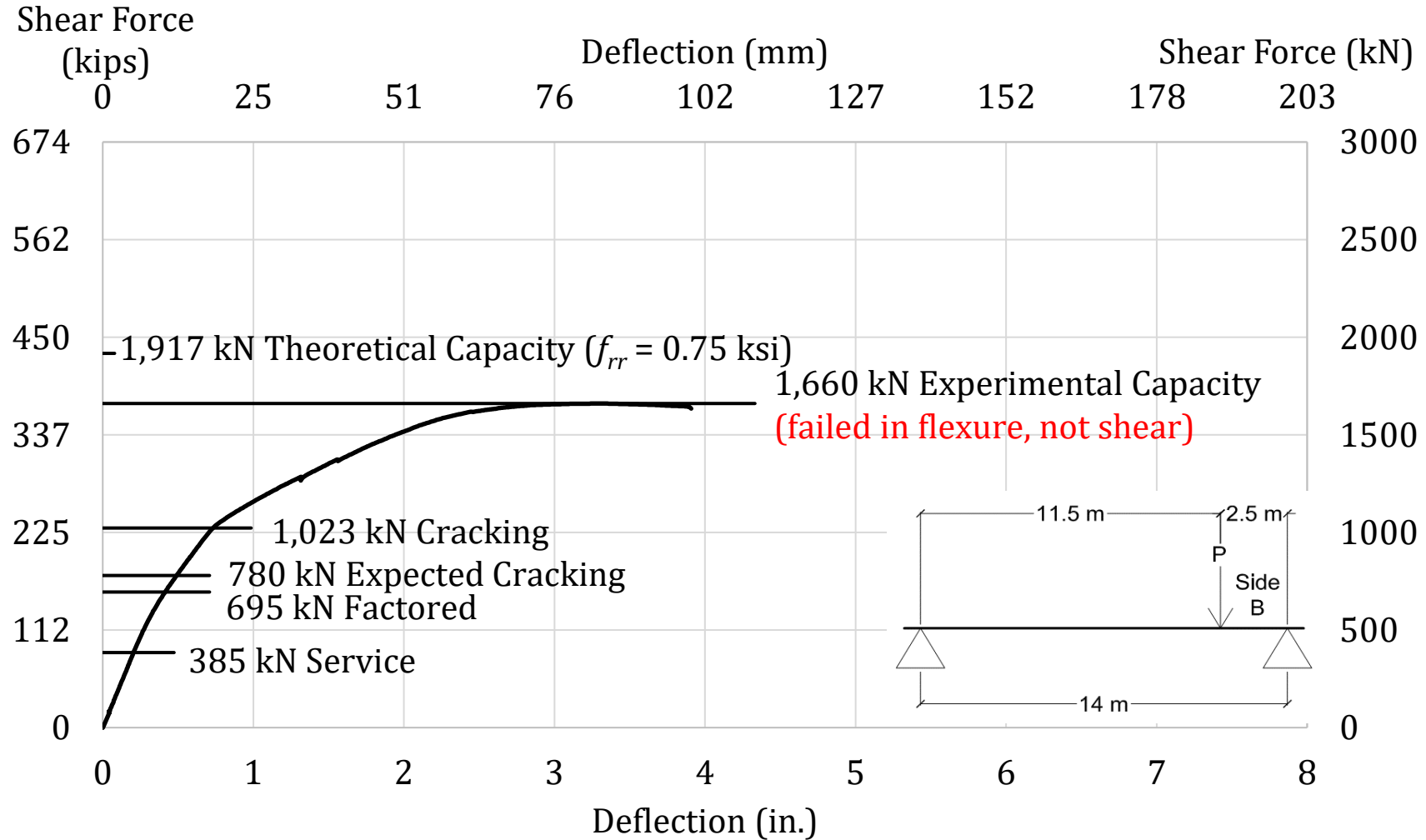
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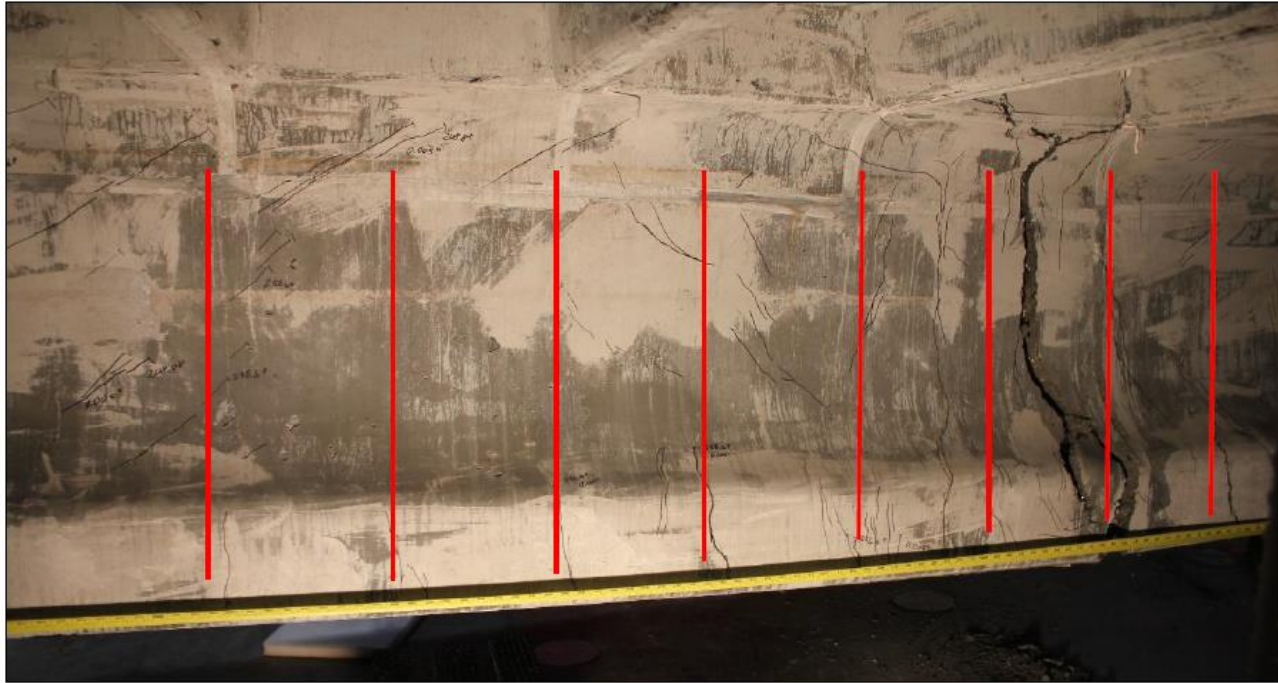


Shear Testing – Side with Stirrups (#5 @ 10")

Source:
NCSU



Shear Testing – Side with Stirrups (#5 @ 10”)



Flexural cracks at each stirrup (red lines = stirrup locations)

Source: NCSU



Close view of failure location showing stirrup

Shear Testing – Side with Stirrups (Video) Source: NCSU

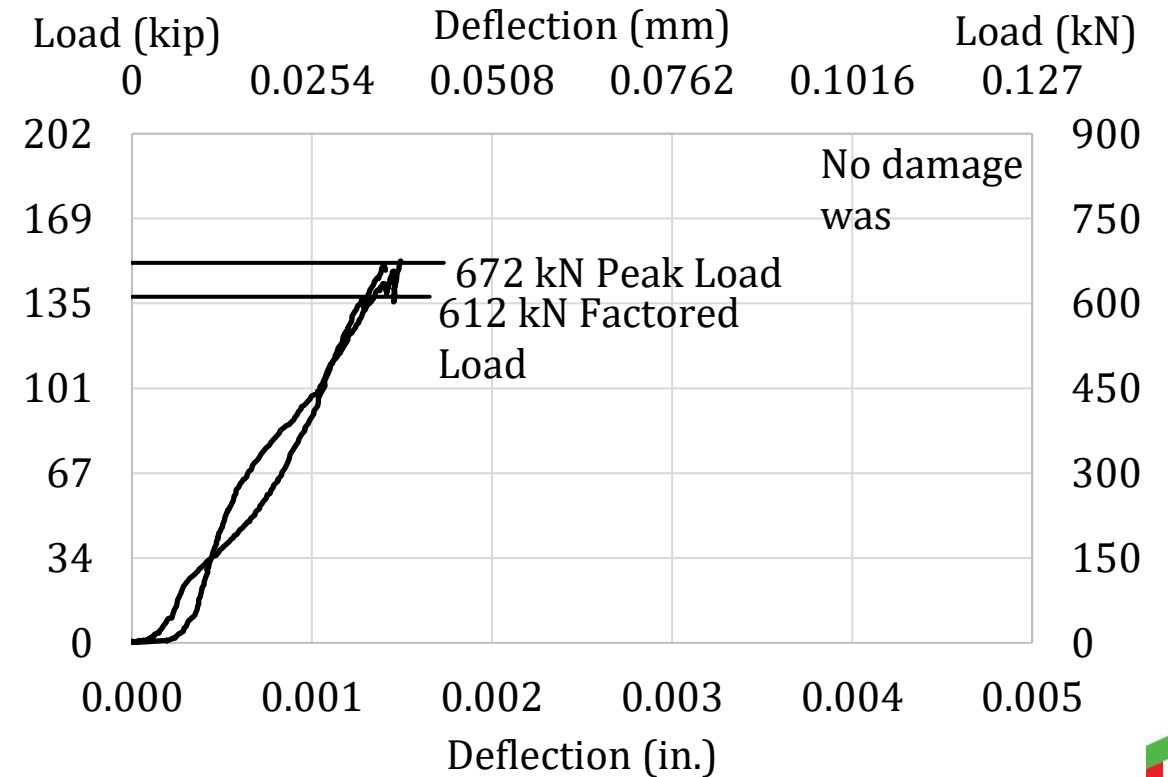
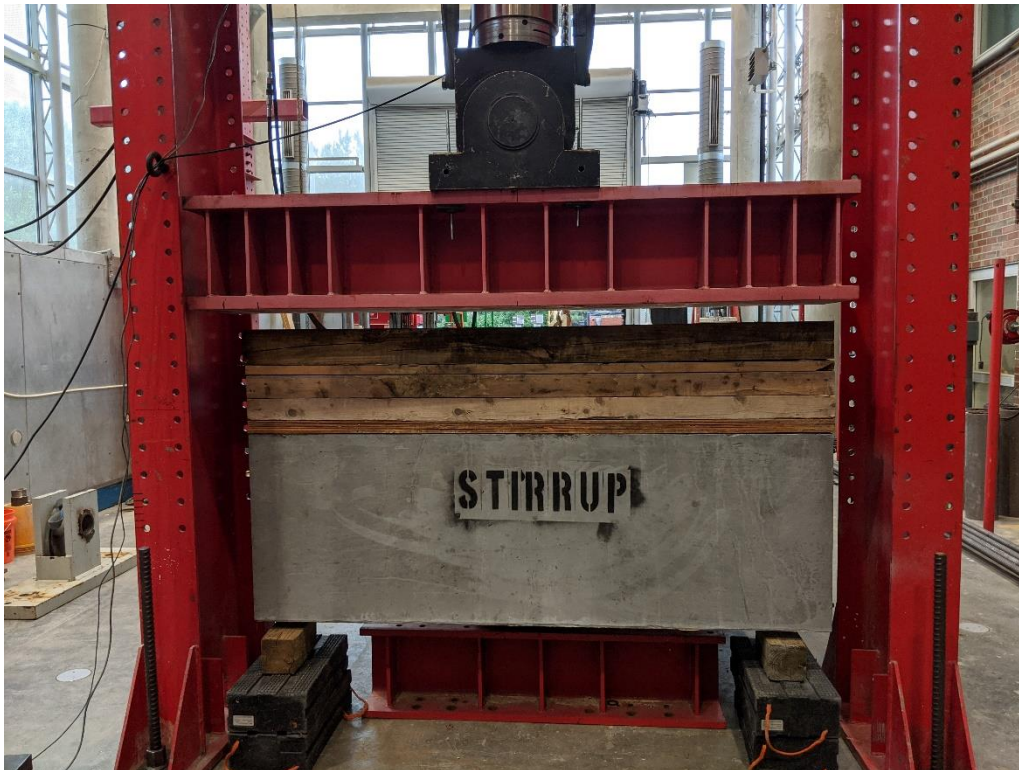


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Semi-Integral Diaphragm Test

Source:
NCSU



University of Nebraska Test Results

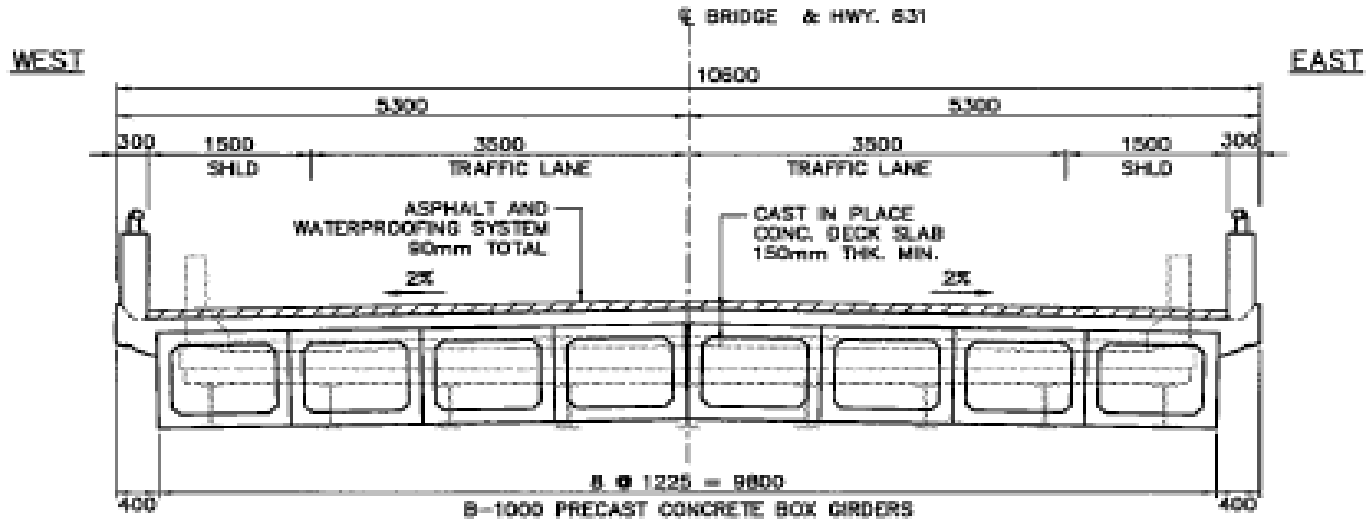
- Please see separate presentation by Drs. Morcous and Tadros.
- Results confirm conservative design.
- Results are consistent with NC State University results.

What is next? Potential New Project in Ontario

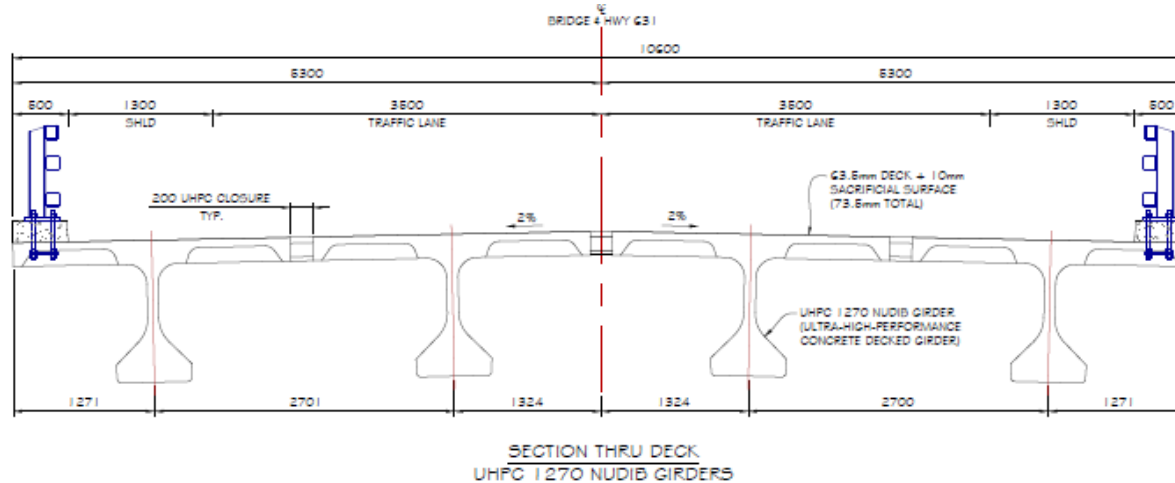
- Converted from conventional concrete adjacent boxes to UHPC DIB

What's next ?

Note: Preliminary details subject to further review if considered by the client

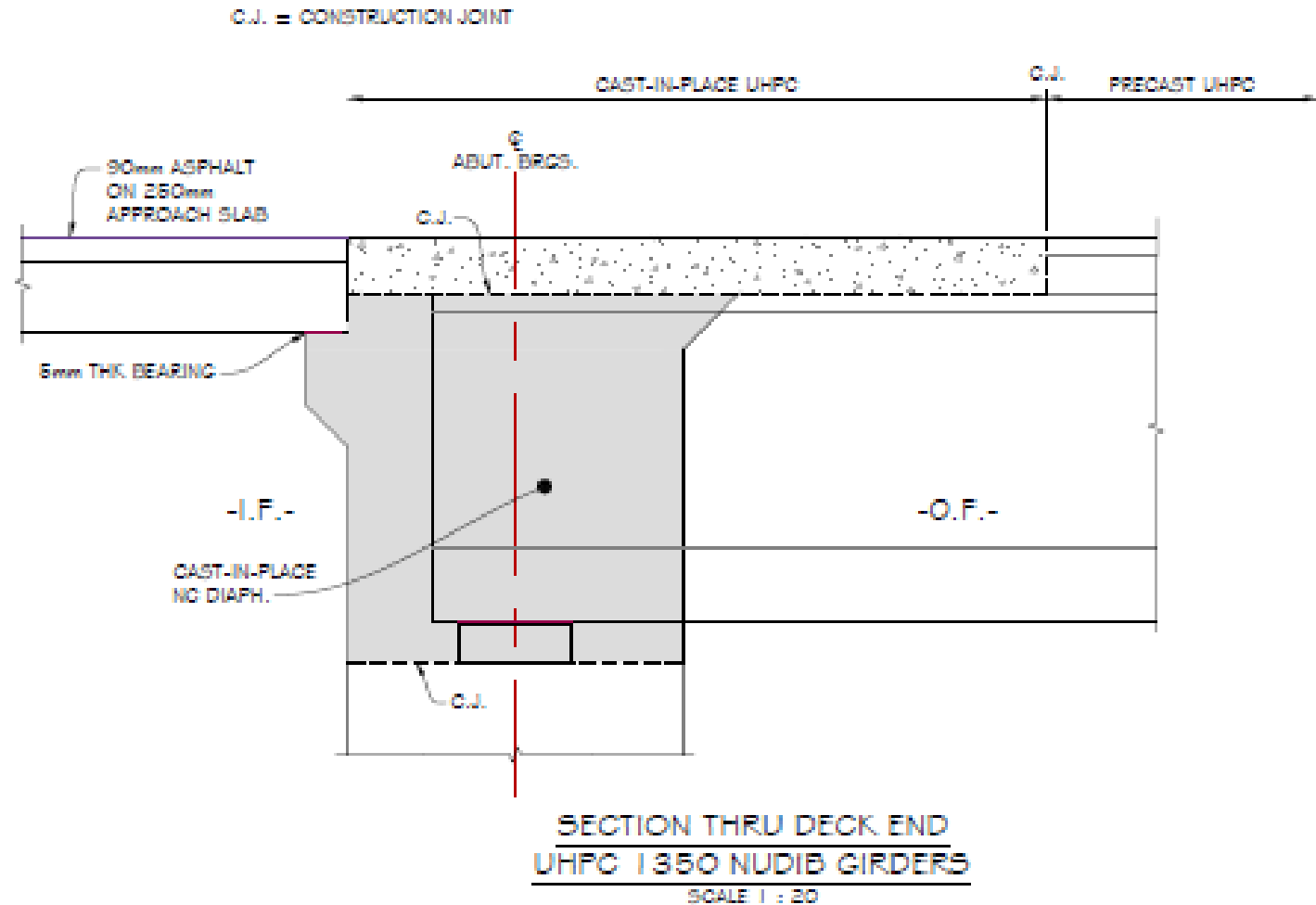


As-tendered design



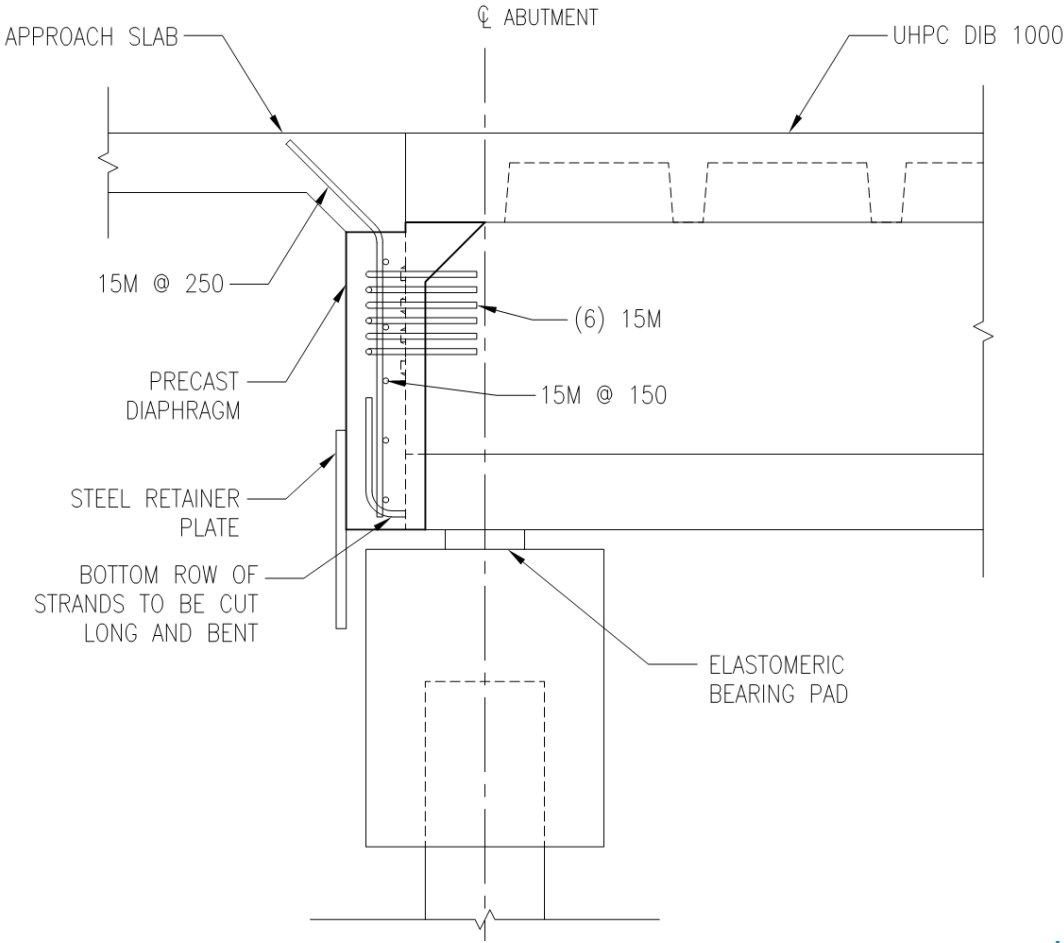
UHPC NUDIB alternative
With TL4 3-tube railing

Integral abutments



Note: Preliminary details subject to further review if considered by the Client

Semi-Integral Diaphragm Option



Conclusions

- Use of precast UHPC is environmentally responsible and cost effective
- Structural products can be produced in current precasting facilities with minor equipment modification
- Quality control must be a top priority
- Excellent structural performance in full scale testing by 2 universities
- Testing demonstrated conservative design.
- Eliminate discrete mild steel shear reinforcement using a narrow 4 inch (100 mm) web

Thank You

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